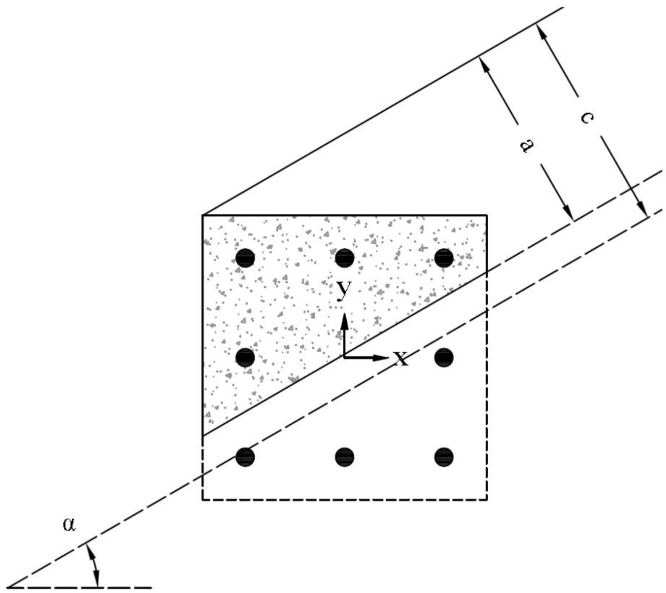
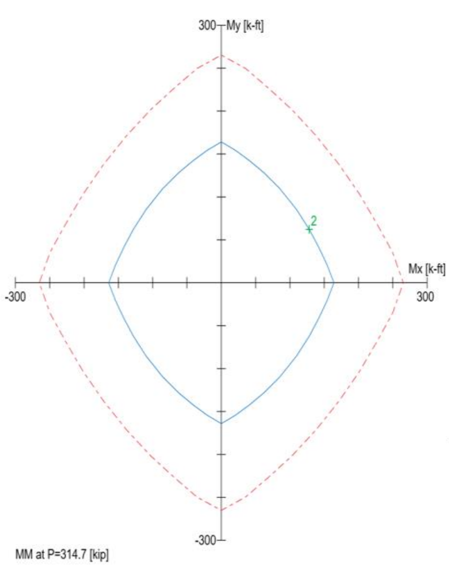
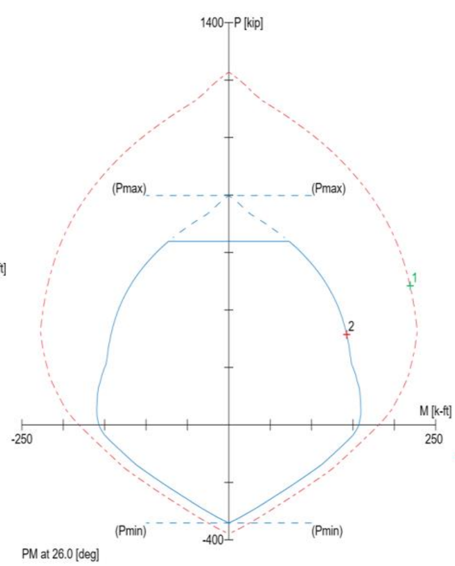
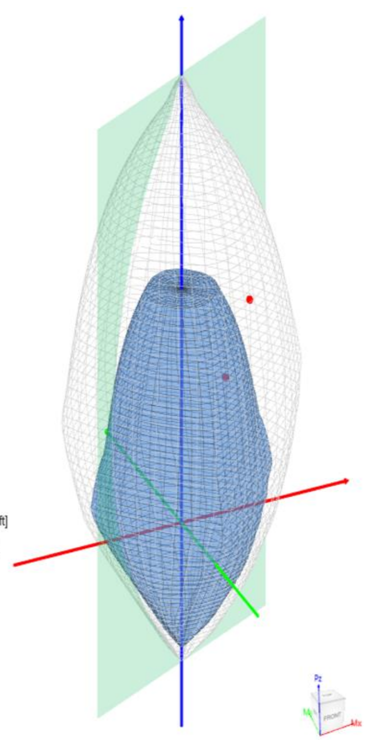
Biaxial Bending Interaction Diagrams for Square Reinforced Concrete Column Design (ACI 318-19)









Biaxial Bending Interaction Diagrams for Square Reinforced Concrete Column Design (ACI 318-19)

Biaxial bending of columns occurs when the loading causes bending simultaneously about both principal axes. The commonly encountered case of such loading occurs in corner columns. Corner and other columns exposed to known moments about each axis simultaneously should be designed for the combined effect of biaxial bending and axial load.

A uniaxial interaction diagram defines the load-moment strength along a single plane of a section under an axial load P and a uniaxial moment M. The biaxial bending resistance of an axially loaded column can be represented schematically as a surface formed by a series of uniaxial interaction curves drawn radially from the P axis. Data for these intermediate curves are obtained by varying the angle of the neutral axis (for assumed strain configurations) with respect to the major axes.

The difficulty associated with the determination of the strength of reinforced columns subjected to combined axial load and biaxial bending is primarily an arithmetic one. The bending resistance of an axially loaded column about a particular skewed axis is determined through iterations involving simple but lengthy calculations. These extensive calculations are compounded when optimization of the reinforcement or cross-section is sought.

This example demonstrates the determination of the design axial load capacity, ϕP_n, and the design ϕM_{nx} and ϕM_{ny} moments corresponding to the following case: The neutral axis position crosses the vertical axis of symmetry of the section (y-axis) at 10 in. below the top of the section, at an angle of 30º counterclockwise from the x-axis of the cross section. The figure below shows the reinforced concrete square column cross section in consideration. The calculated values of the column axial strength and biaxial bending strength are compared with the values from the reference and the exact values from spColumn engineering software program from StructurePoint. The steps to develop the three-dimensional failure surface (interaction diagram) using spColumn will be shown in detail as well.

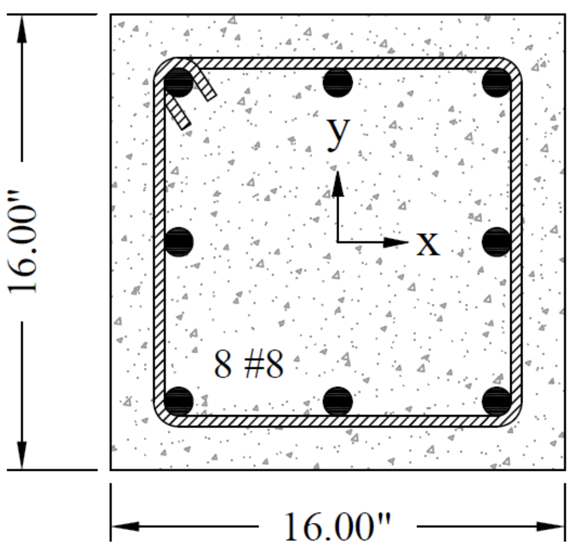


Figure 1 – Reinforced Concrete Column Cross-Section

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Code

Building Code Requirements for Structural Concrete (ACI 318-19) and Commentary (ACI 318R-19)

References

- Reinforced Concrete Mechanics and Design, 8th Edition, 2021, James Wight, Pearson, Example 11-5
- Notes on ACI 318-11 Building Code Requirements for Structural Concrete, Twelfth Edition, 2013 Portland Cement Association
- spColumn Engineering Software Program Manual v10.00, STRUCTUREPOINT, 2021
- "Biaxial Bending Interaction Diagrams for Rectangular Reinforced Concrete Column Design (ACI 318-19)" Design Example, STRUCTUREPOINT, 2022
- "Biaxial Bending Interaction Diagrams for C-Shaped Concrete Core Wall Design (ACI 318-19)" Design Example, STRUCTUREPOINT, 2022
- "Biaxial Bending Interaction Diagrams for Spiral Reinforced Circular Concrete Column Design (ACI 318-19)" Design Example, STRUCTUREPOINT, 2022
- "Manual Design Procedure for Columns and Walls with Biaxial Bending (ACI 318-11/14/19)" Design Example, STRUCTUREPOINT, 2022

Design Data

f_c' = 4,000 psi

f_y = 60,000 psi

Cover = 2.4 in.

Column dimensions and reinforcement locations are shown in following figure.

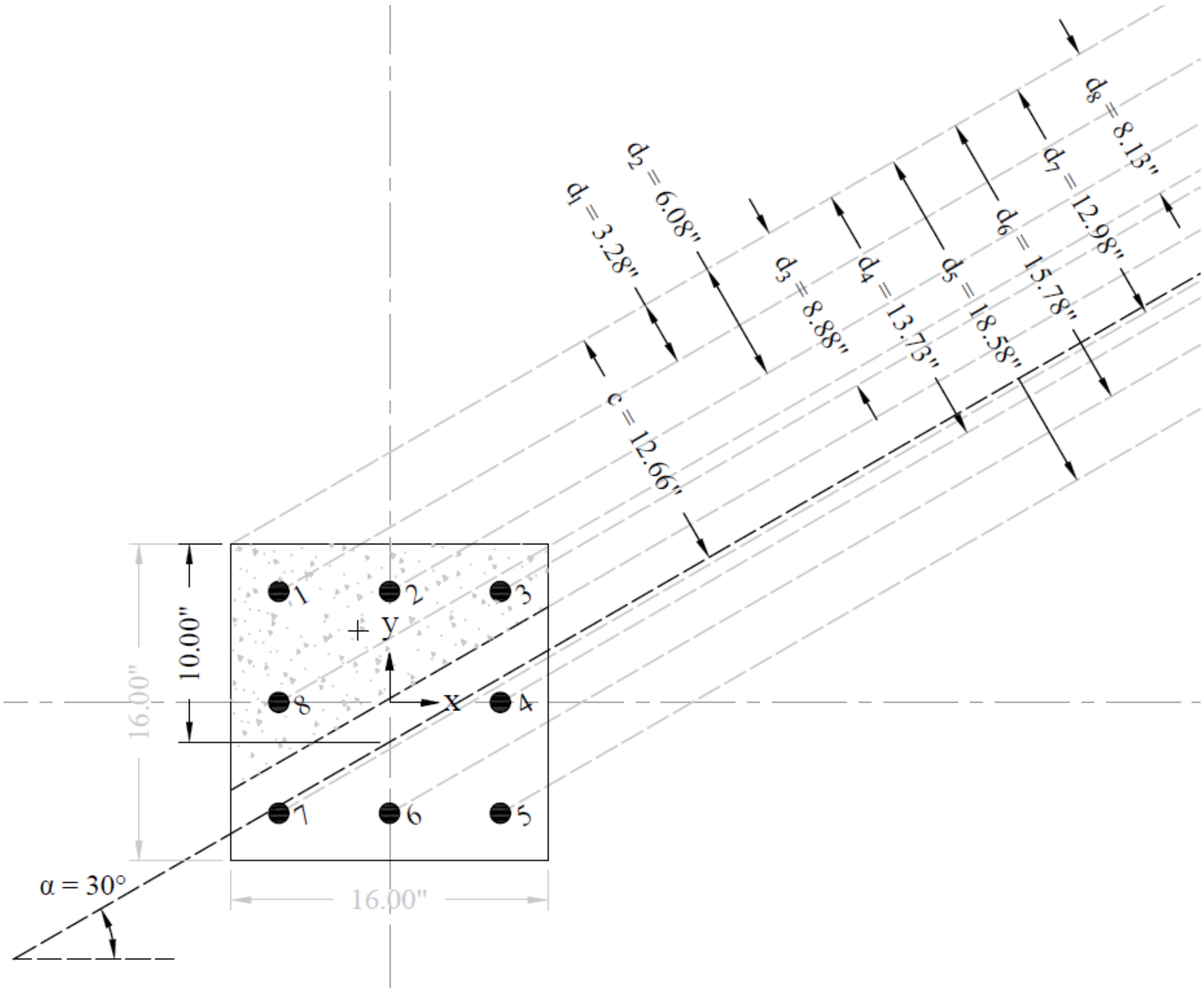

Figure 2 – Reinforced Concrete Column Cross-Section and Reinforcement Locations

Solution

In a reinforced concrete column, the determination of the nominal axial load capacity, P_n, and the nominal M_{nx} and M_{ny} moments involves a trial-and-error process for calculating the neutral axis depth and angle α. The reference provided the neutral axis depth and angle as an input (The neutral axis position crosses the vertical axis of symmetry of the section at 10 in. leading to c = 12.66 in. and an angle of α = 30.0º) for illustration.

The steps to calculate biaxial flexural strength of a reinforced concrete column for a given nominal axial strength and moment ratio of biaxial bending moments is discussed in details in "Biaxial Bending Interaction Diagrams for Rectangular Reinforced Concrete Column Design (ACI 318-19)" design example.

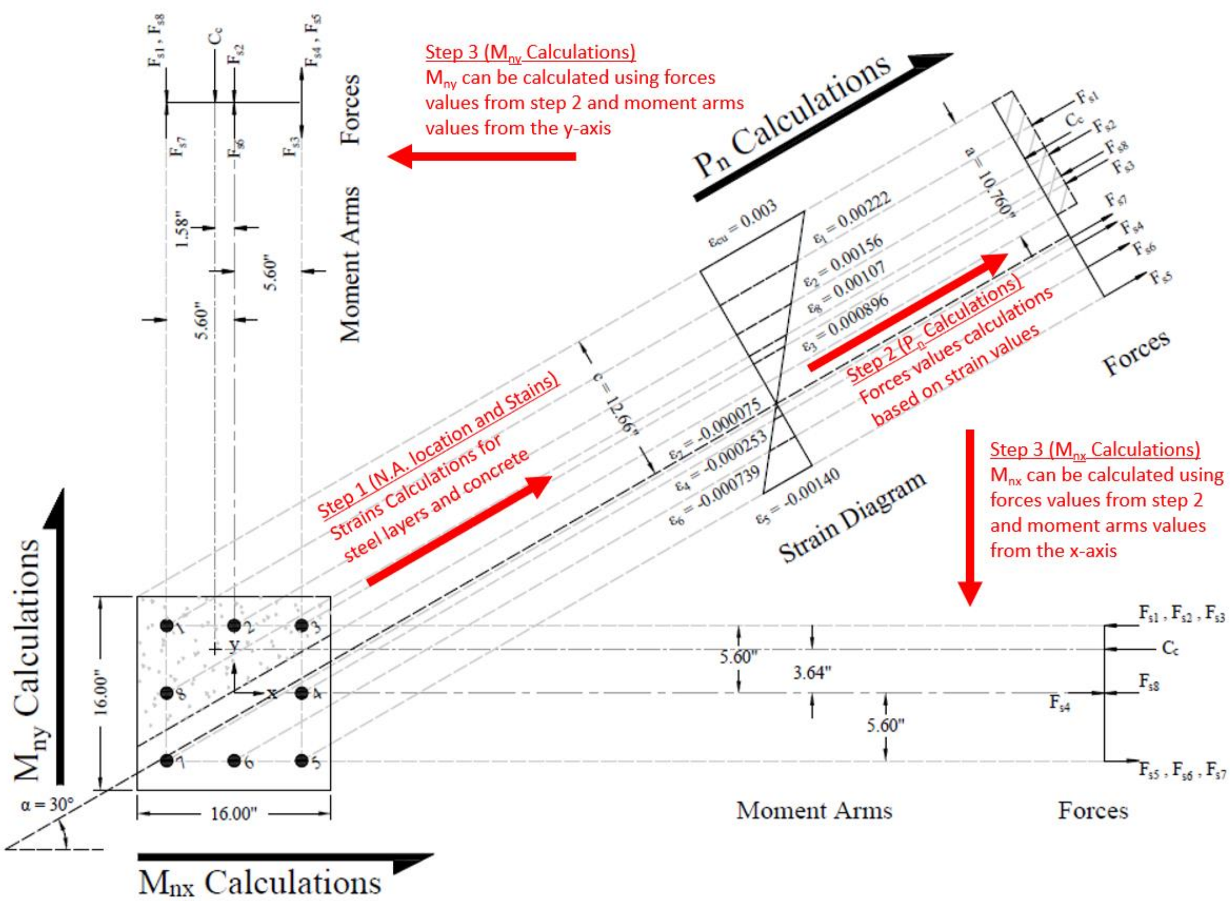

Figure 3 – Nominal Axial Load and Biaxial Flexural Strength Calculation Methods for a Reinforced Concrete Column.

1. Concrete Column Biaxial Strength Calculations

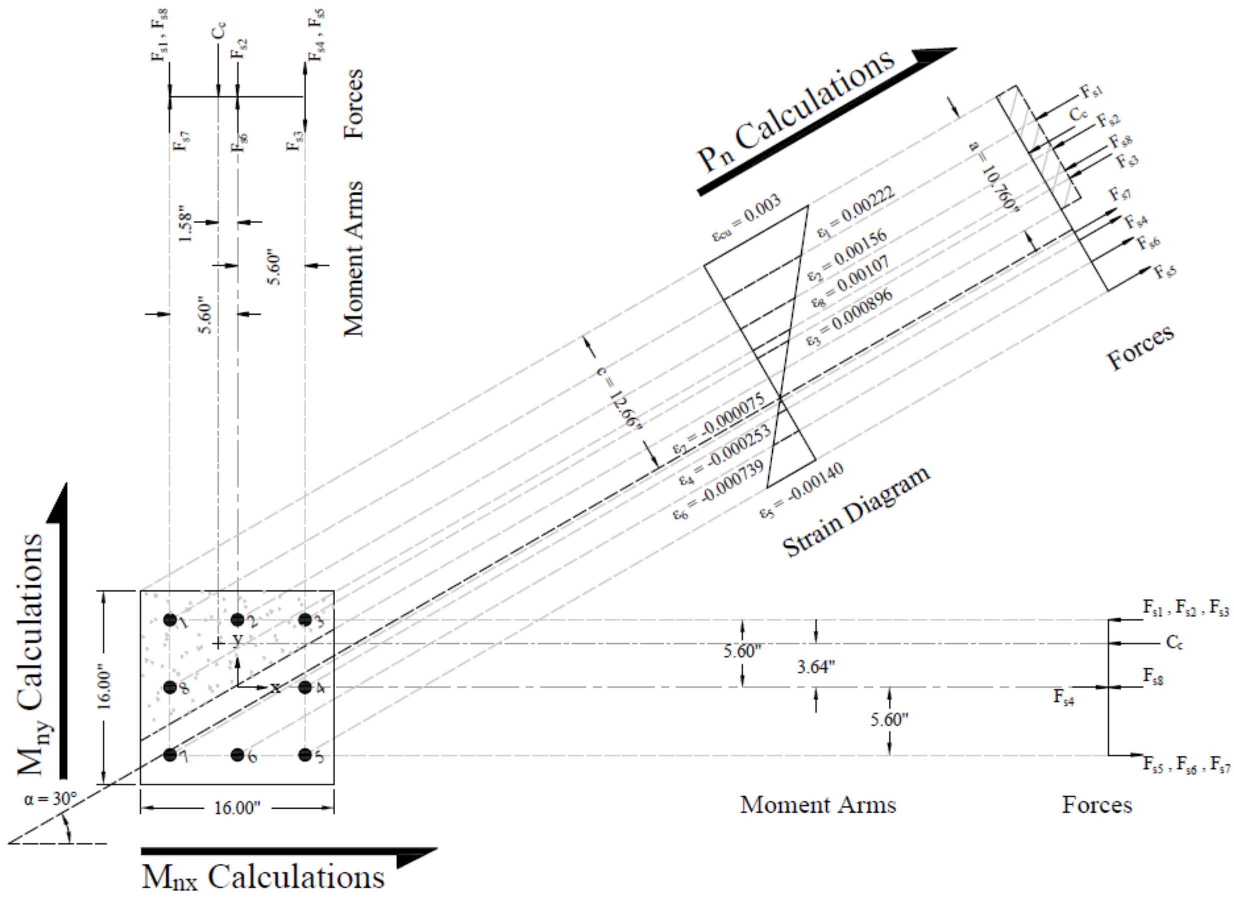

Figure 4 – Strains, Forces, and Moment Arms Diagram

1.1. Location of Neutral Axis and Concrete Compression Force

The trial-and-error process for calculating the neutral axis depth and angle α is not required in this example since these values are given by the reference (c = 12.66 in. and α = 30.0º). Where *c* is the distance from the fiber of maximum compressive strain to the neutral axis and α is the angle of the neutral axis.

ACI 318-19 (22.2.2.4.2)

$$\varepsilon_y = \frac{f_y}{E_s} = \frac{60}{29,000} = 0.00207$$

$$\varepsilon_{s5} = (c - d_5) \times \frac{\varepsilon_{cu}}{c} = (12.66 - 18.578) \times \frac{0.003}{12.66} = -0.00140 \text{ (Tension)} < \varepsilon_y \rightarrow \text{reinforcement has not yielded}$$

$$\therefore \phi = 0.65$$ ***ACI 318-19 (Table 21.2.2)***

$$a = \beta_1 \times c = 0.85 \times 12.66 = 10.761 \text{ in.}$$ ***ACI 318-19 (22.2.2.4.1)***

$$\varepsilon_{cu} = 0.003$$ ***ACI 318-19 (22.2.2.1)***

Where:

a = Depth of equivalent rectangular stress block

$$\beta_1 = 0.85 - \frac{0.05 \times (f'_c - 4000)}{1000} = 0.85 - \frac{0.05 \times (4000 - 4000)}{1000} = 0.85$$ ***ACI 318-19 (Table 22.2.2.4.3)***

$$C_c = 0.85 \times f'_c \times A_{comp} = 0.85 \times 4000 \times 124.88 = 424.59 \text{ kip (Compression)}$$ ***ACI 318-19 (22.2.2.4.1)***

Where (see the following figure):

$$A_{comp} = A_1 + A_2 = (3.19 \times 16) + \left(\frac{1}{2} \times 9.24 \times 16\right) = 124.88 \text{ in.}^2$$

$$\bar{x} = \left(\frac{A_1 \times \overline{x_1} + A_2 \times \overline{x_2}}{A_1 + A_2}\right) - 8.00 = \left(\frac{51.04 \times 8.00 + 73.84 \times 5.33}{51.04 + 73.84}\right) - 8.00 = -1.58 \text{ in.}$$

$$\bar{y} = \left(\frac{A_1 \times \overline{y_1} + A_2 \times \overline{y_2}}{A_1 + A_2}\right) - 4.42 = \left(\frac{51.04 \times 10.825 + 73.84 \times 6.153}{51.04 + 73.84}\right) - 4.42 = 3.64 \text{ in.}$$

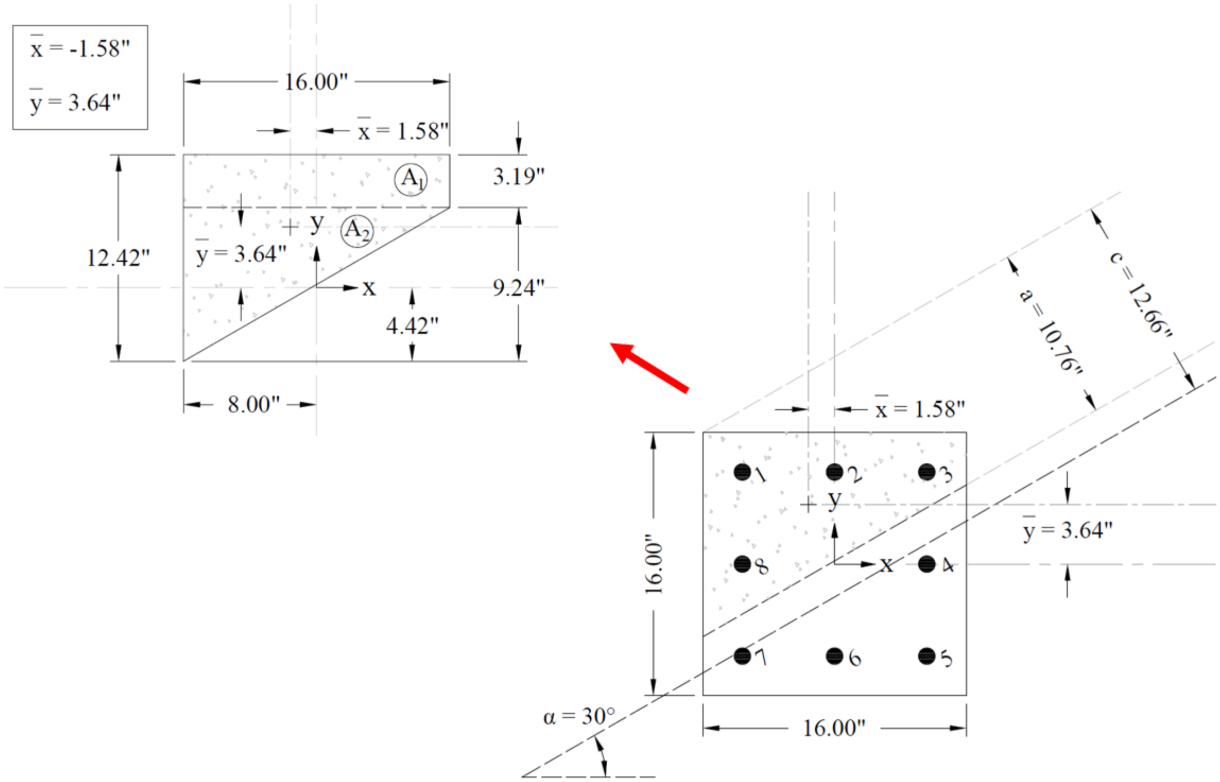

Figure 5 – Cracked Concrete Column Section Centroid Calculations

1.2. Strains and Forces Determination in Reinforcement Layers

The following shows the calculations of forces in the reinforcement layers with the extreme tension (at bar 5) and extreme compression (at bar 1) strains. The calculations for the rest of layers are shown the table at the end of this section.

For extreme tension reinforcement layer (at bar 5):

$$\varepsilon_{s5} = -0.00140 \text{ (Tension)} < \varepsilon_y \rightarrow \text{reinforcement has not yielded}$$

$$\therefore\ f_{s5} = \varepsilon_{s5} \times E_s = -0.00140 \times 29000000 = -40669 \text{ psi}$$

$$F_{s5} = f_{s5} \times A_{s5} = -40669 \times (1 \times 0.79) = -32.13 \text{ kip} \quad \text{(Tension)}$$

For extreme compression reinforcement layer (at bar 1):

$$\varepsilon_{s1} = (c - d_1) \times \frac{\varepsilon_{cu}}{c} = (12.66 - 3.278) \times \frac{0.003}{12.66} = 0.00222 \text{ (Compression)} > \varepsilon_y \rightarrow \text{reinforcement has yielded}$$

$$\therefore\ f_{s1} = f_y = 60000 \text{ psi}$$

The area of the reinforcement in this layer is included in the area used to compute C_c (a = 10.76 in. > d_1 = 3.28 in.). As a result, it is necessary to subtract $0.85f_c'$ from f_{s1} before computing F_{s1}:

$$\mathrm{F}_{s1} = f_{s1} \times A_{s1} = (60000 - 0.85 \times 4000) \times (1 \times 0.79) = 44.71 \text{ kip (Compression)}$$

The same procedure shown above can be repeated to calculate the forces in the remaining reinforcement locations, results are summarized in the following table:

Table 1 - Strains, internal force resultants and Moments

Location	d, in.	ε, in./in.	f_s, psi	F_s, kip	C_c, kip	Moment arm (x), in.	M_y, kip-ft	Moment arm (y), in.	M_x, kip-ft
Concrete	---	0.00300	---	---	424.59	1.58	55.90	3.64	128.79
Bar 1	3.278	0.00222	60000	44.71*	---	5.60	20.87	5.60	20.87
Bar 2	6.078	0.00156	45232	33.05*	---	0.00	0.00	5.60	15.42
Bar 3	8.878	0.00090	25990	17.85*	---	-5.60	-8.33	5.60	8.33
Bar 4	13.728	-0.00025	-7339	-5.8	---	-5.60	2.71	0.00	0.00
Bar 5	18.578	-0.00140	-40669	-32.13	---	-5.60	14.99	-5.60	14.99
Bar 6	15.778	-0.00074	-21427	-16.93	---	0.00	0.00	-5.60	7.90
Bar 7	12.978	-0.00008	-2185	-1.73	---	5.60	-0.81	-5.60	0.81
Bar 8	8.128	0.00107	31144	21.92*	---	5.60	10.23	0.00	0.00
Axial Force and Biaxial Bending Moments Capacities			P_n, kip	485.54		M_{ny}, kip-ft	95.56	M_{nx}, kip-ft	197.11
			ϕP_n, kip	315.60		ϕM_{ny}, kip-ft	62.12	ϕM_{nx}, kip-ft	128.12

* The area of the reinforcement in this layer has been included in the area used to compute C_c. As a result, $0.85f_c'$ is subtracted from f_s in the computation of F_s.

1.3. Calculation of P_n, M_{nx} and M_{ny}

$$P_n = C_c + \sum F_s$$ (+) = Compression (-) = Tension

$$\phi P_n = \phi \times P_n = 0.65 \times P_n$$

$$M_{ny} = C_c \times \left(\frac{b}{2} - \bar{x}_c\right) + \sum_{i=1}^{n=8} \left(F_{si} \times \left(\frac{b}{2} - x_i\right)\right)$$ (+) = Counter Clockwise (-) = Clockwise

$$\phi M_{ny} = \phi \times M_{ny} = 0.65 \times M_{ny}$$

$$M_{nx} = C_c \times \left(\frac{h}{2} - \bar{y}_c\right) + \sum_{i=1}^{n=8} \left(F_{si} \times \left(\frac{h}{2} - y_i\right)\right)$$ (+) = Counter Clockwise (-) = Clockwise

$$\phi M_{nx} = \phi \times M_{nx} = 0.65 \times M_{nx}$$

2. Column Biaxial Bending Interaction Diagram – spColumn Software

spColumn is a StructurePoint software program that performs the analysis and design of reinforced concrete sections subjected to axial force combined with uniaxial or biaxial bending. Using the provisions of the Strength Design Method and Unified Design Provisions, slenderness considerations are used for moment magnification due to second order effect (P-Delta) for sway and non-sway frames.

For biaxial runs, the values of maximum compressive axial load capacity and maximum tensile load capacity are computed. These two values set the range within which the moment capacities are computed for a predetermined number of axial load values. For each level of axial load, the section is rotated in 10-degree increments from 0 degrees to 360 degrees and the M_x and M_y moment capacities are computed. Thus, for each level of axial load, an M_x-M_y contour is developed. Repeating this for the entire range of axial loads, the three-dimensional failure surface is computed. A three-dimensional visualization of the resulting entire nominal and factored failure surface is provided to support enhanced understanding of the section capacity.

The **"biaxial"** feature allows the user to investigate the P-M interaction diagrams, the M_x-M_y moment contour plots, as well as the 3D failure surface for even the most irregular column and shear wall sections quickly, simply, and accurately.

For this column section, we ran in investigation mode with **"biaxial"** option for "Run Axis" using the ACI 318-19.

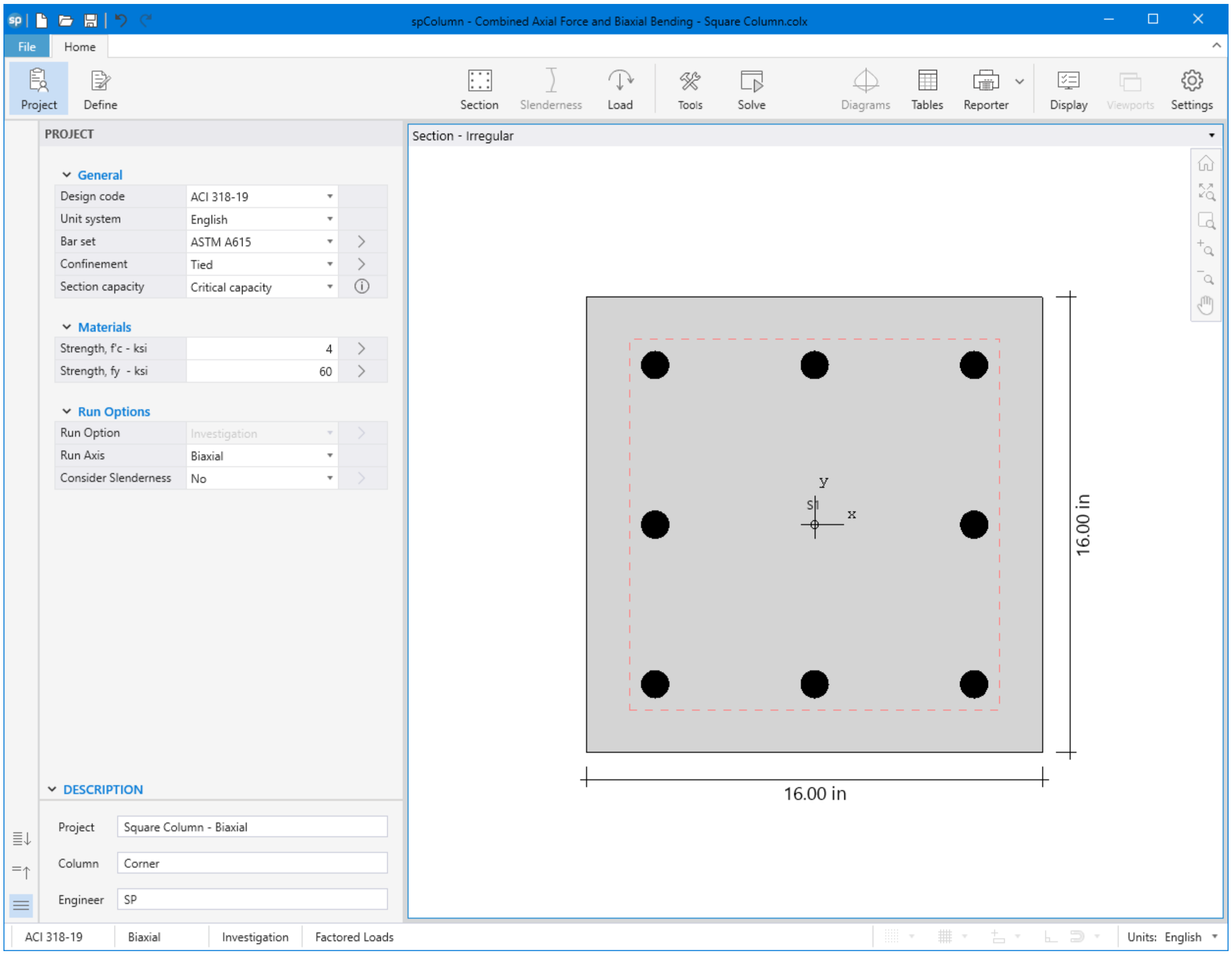

Figure 6 – spColumn Interface

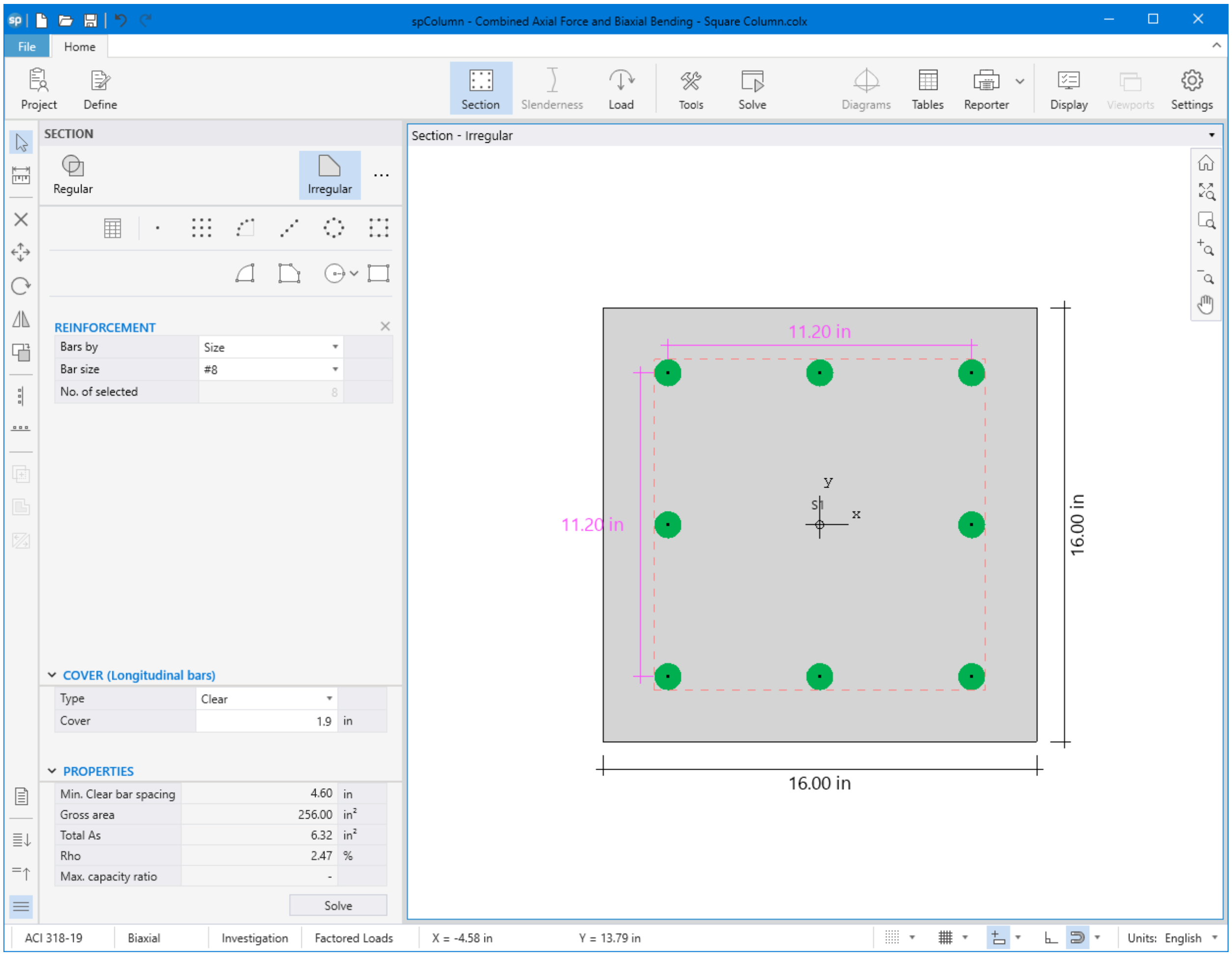

Figure 7 – spColumn Model Editor

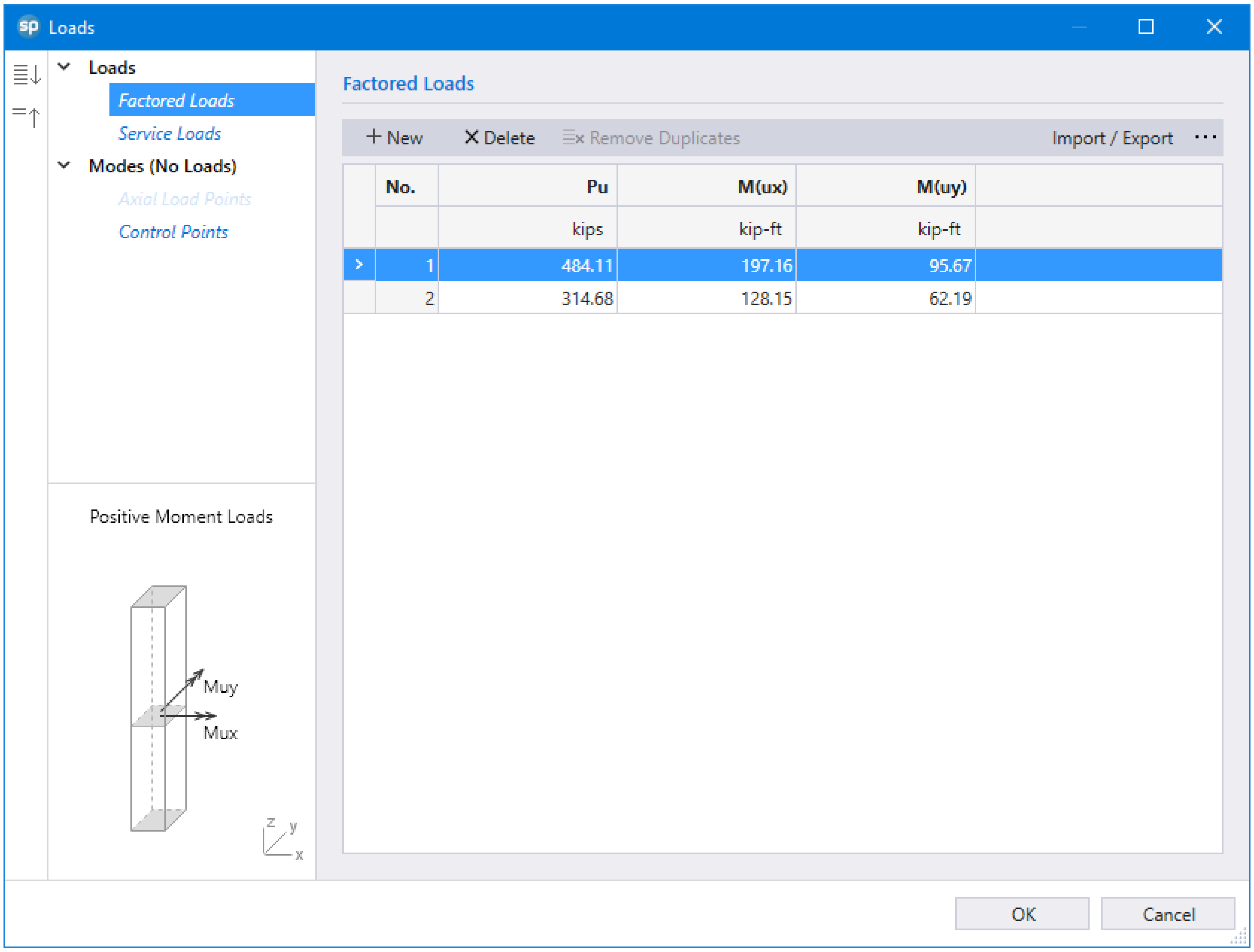

Figure 8 – Defining Loads / Modes (spColumn)

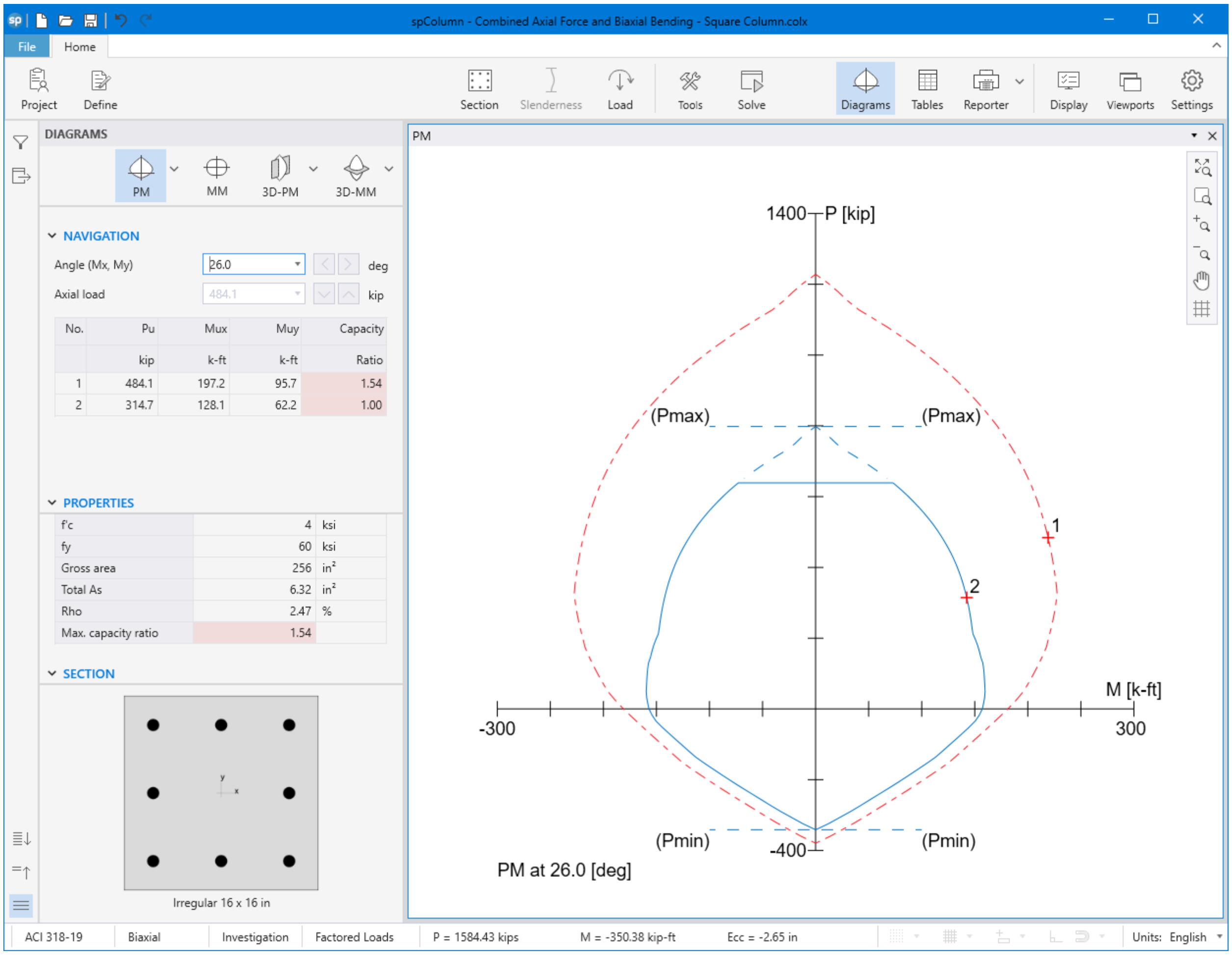

Figure 9 – Column Section Interaction Diagram at 26º (spColumn)

spColumn v10.00 (TM)
Computer program for the Strength Design of Reinforced Concrete Sections

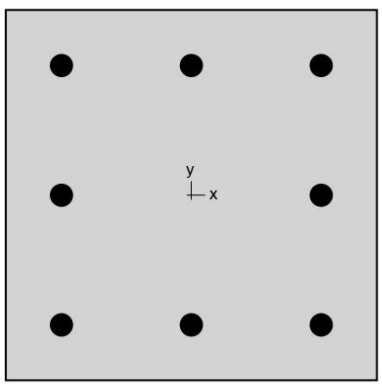

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Figure 1: Column section 4

1. General Information

File Name	...\Combined Axial Force and Biaxial Bending -...
Project	Square Column - Biaxial
Column	Corner
Engineer	SP
Code	ACI 318-19
Bar Set	ASTM A615
Units	English
Run Option	Investigation
Run Axis	Biaxial
Slenderness	Not Considered
Column Type	Structural
Capacity Method	Critical capacity

2. Material Properties

2.1. Concrete

Type	Standard	
f'_c	4	ksi
E_c	3605	ksi
f_c	3.4	ksi
ε_u	0.003	in/in
β_1	0.85	

2.2. Steel

Type	Standard	
f_y	60	ksi
E_s	29000	ksi
ε_{ty}	0.00206897	in/in

3. Section

3.1. Shape and Properties

Type	Irregular	
A_g	256	in²
I_x	5461.33	in⁴
I_y	5461.33	in⁴
r_x	4.6188	in
r_y	4.6188	in
X_o	0	in
Y_o	0	in

3.2. Section Figure

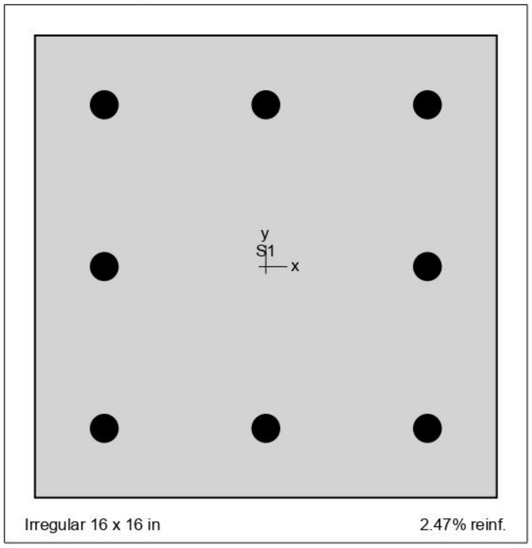

Figure 1: Column section

3.3. Solids

3.3.1. S1

Points	X	Y	Points	X	Y	Points	X	Y
	in	in		in	in		in	in
1	-8.0	8.0	2	-8.0	-8.0	3	8.0	-8.0
4	8.0	8.0						

4. Reinforcement

4.1. Bar Set: ASTM A615

Bar	Diameter	Area	Bar	Diameter	Area	Bar	Diameter	Area
	in	in²		in	in²		in	in²
#3	0.38	0.11	#4	0.50	0.20	#5	0.63	0.31
#6	0.75	0.44	#7	0.88	0.60	#8	1.00	0.79
#9	1.13	1.00	#10	1.27	1.27	#11	1.41	1.56
#14	1.69	2.25	#18	2.26	4.00			

4.2. Confinement and Factors

Confinement type	Tied
For #10 bars or less	#3 ties
For larger bars	#4 ties
Capacity Reduction Factors	
Axial compression, (a)	0.8
Tension controlled ϕ, (b)	0.9
Compression controlled ϕ, (c)	0.65

4.3. Arrangement

Pattern	Irregular
Bar layout	---
Cover to	---
Clear cover	---
Bars	---
Total steel area, A_s	6.32 in²
Rho	2.47 %
Minimum clear spacing	4.60 in

4.4. Bars Provided

Area	X	Y	Area	X	Y	Area	X	Y
in²	in	in	in²	in	in	in²	in	in
0.79	-5.6	-5.6	0.79	-5.6	0.0	0.79	-5.6	5.6
0.79	0.0	-5.6	0.79	0.0	5.6	0.79	5.6	-5.6
0.79	5.6	0.0	0.79	5.6	5.6			

5. Control Points

About	Point	P	X-Moment	Y-Moment	NA Depth	d_t Depth	ε_t	ϕ
		kip	k-ft	k-ft	in	in		
X	@ Max compression	798.3	0.00	0.00	43.82	13.60	-0.00207	0.65000
X	@ Allowable comp.	638.6	75.39	0.00	16.37	13.60	-0.00051	0.65000
X	@ f_s = 0.0	529.3	116.31	0.00	13.60	13.60	0.00000	0.65000
X	@ f_s = 0.5 f_y	360.1	156.03	0.00	10.11	13.60	0.00103	0.65000
X	@ Balanced point	237.2	176.14	0.00	8.05	13.60	0.00207	0.65000
X	@ Tension control	100.7	204.44	0.00	5.06	13.60	0.00507	0.90000
X	@ Pure bending	0.0	169.84	0.00	3.72	13.60	0.00797	0.90000
X	@ Max tension	-341.3	0.00	0.00	0.00	13.60	9.99999	0.90000
Y	@ Max compression	798.3	0.00	0.00	43.82	13.60	-0.00207	0.65000
Y	@ Allowable comp.	638.6	0.00	75.39	16.37	13.60	-0.00051	0.65000
Y	@ f_s = 0.0	529.3	0.00	116.31	13.60	13.60	0.00000	0.65000
Y	@ f_s = 0.5 f_y	360.1	0.00	156.03	10.11	13.60	0.00103	0.65000
Y	@ Balanced point	237.2	0.00	176.14	8.05	13.60	0.00207	0.65000
Y	@ Tension control	100.7	0.00	204.44	5.06	13.60	0.00507	0.90000
Y	@ Pure bending	0.0	0.00	169.84	3.72	13.60	0.00797	0.90000
Y	@ Max tension	-341.3	0.00	0.00	0.00	13.60	9.99999	0.90000
-X	@ Max compression	798.3	0.00	0.00	43.82	13.60	-0.00207	0.65000
-X	@ Allowable comp.	638.6	-75.39	0.00	16.37	13.60	-0.00051	0.65000
-X	@ f_s = 0.0	529.3	-116.31	0.00	13.60	13.60	0.00000	0.65000
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-X	@ Pure bending	0.0	-169.84	0.00	3.72	13.60	0.00797	0.90000
-X	@ Max tension	-341.3	0.00	0.00	0.00	13.60	9.99999	0.90000
-Y	@ Max compression	798.3	0.00	0.00	43.82	13.60	-0.00207	0.65000
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-Y	@ Tension control	100.7	0.00	-204.44	5.06	13.60	0.00507	0.90000
-Y	@ Pure bending	0.0	0.00	-169.84	3.72	13.60	0.00797	0.90000
-Y	@ Max tension	-341.3	0.00	0.00	0.00	13.60	9.99999	0.90000

STRUCTUREPOINT - spColumn v10.00 (TM)
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6. Factored Loads and Moments with Corresponding Capacity Ratios

NOTE: Calculations are based on "Critical Capacity" Method.

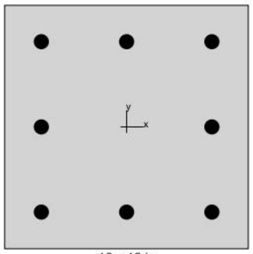
No.	Demand			Capacity			Parameters at Capacity			Capacity
	P_u	M_{ux}	M_{uy}	ϕP_n	ϕM_{nx}	ϕM_{ny}	NA Depth	ε_t	ϕ	Ratio
	kip	k-ft	k-ft	kip	k-ft	k-ft	in			
1	484.11	197.16	95.67	357.36	124.69	60.51	13.40	0.00116	0.650	1.54 #
2	314.68	128.15	62.19	314.68	128.15	62.19	12.64	0.00141	0.650	1.00 #

Section capacity exceeded. Revise design!

Two factored loads are applied to locate the nominal (point 1) and design (point 2) capacities of the section. In both points, the capacity ratio is calculated based on the design capacity causing point 1 to show 54% beyond design capacity.

7. Diagrams

7.1. PM at θ=26 [deg]



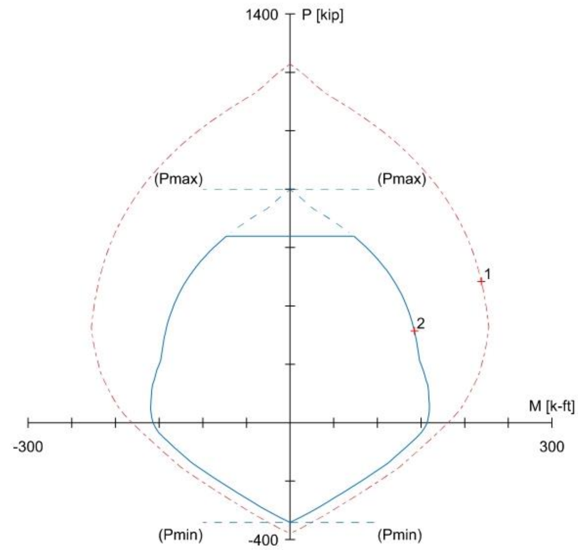
16 x 16 in

General Information	
Project	Square Column - Biaxial
Column	Corner
Engineer	SP
Code	ACI 318-19
Bar Set	ASTM A615
Units	English
Run Option	Investigation
Run Axis	Biaxial
Slenderness	Not Considered
Column Type	Structural
Capacity Method	Critical capacity

Materials	
f'_c	4 ksi
E_c	3605 ksi
f_y	60 ksi
E_s	29000 ksi

Section	
Type	Irregular
A_g	256 in²
I_x	5461.33 in⁴
I_y	5461.33 in⁴

Reinforcement	
Pattern	Irregular
Bar layout	---
Cover to	---
Clear cover	---
Bars	---
Confinement type	Tied
Total steel area, A_s	6.32 in²
Rho	2.47 %
Min. clear spacing	4.60 in

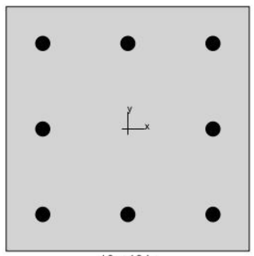


PM at 26.0 [deg]

No.	P_u	M_{ux}	M_{uy}	ϕP_n	ϕM_{nx}	ϕM_{ny}	Capacity
	kip	k-ft	k-ft	kip	k-ft	k-ft	Ratio
1	484.1	197.2	95.7	357.36	124.69	60.51	1.54
2	314.7	128.1	62.2	314.68	128.15	62.19	1.00

Max. Capacity Ratio: 1.54

7.2. MM at P=484 [kip]



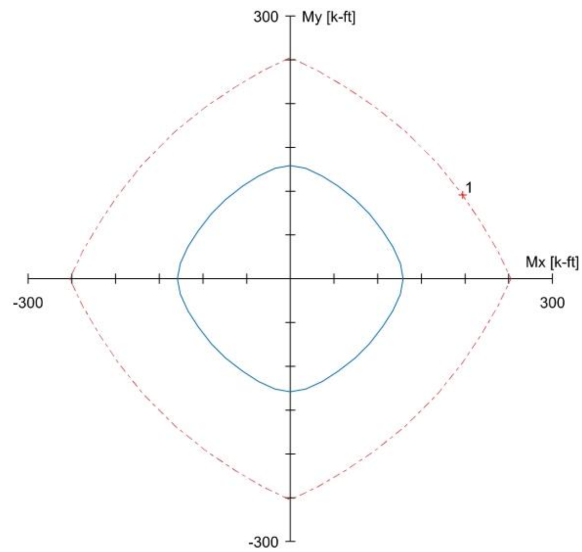
16 x 16 in

General Information	
Project	Square Column - Biaxial
Column	Corner
Engineer	SP
Code	ACI 318-19
Bar Set	ASTM A615
Units	English
Run Option	Investigation
Run Axis	Biaxial
Slenderness	Not Considered
Column Type	Structural
Capacity Method	Critical capacity

Materials	
f'_c	4 ksi
E_c	3605 ksi
f_y	60 ksi
E_s	29000 ksi

Section	
Type	Irregular
A_g	256 in²
I_x	5461.33 in⁴
I_y	5461.33 in⁴

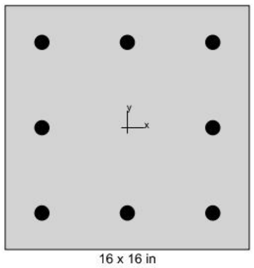
Reinforcement	
Pattern	Irregular
Bar layout	---
Cover to	---
Clear cover	---
Bars	---
Confinement type	Tied
Total steel area, A_s	6.32 in²
Rho	2.47 %
Min. clear spacing	4.60 in



No.	P_u kip	M_{ux} k-ft	M_{uy} k-ft	ϕP_n kip	ϕM_{nx} k-ft	ϕM_{ny} k-ft	Capacity Ratio
1	484.1	197.2	95.7	357.36	124.69	60.51	1.54

Max. Capacity Ratio: 1.54

7.3. MM at P=314.7 [kip] [User]



General Information

Project	Square Column - Biaxial
Column	Corner
Engineer	SP
Code	ACI 318-19
Bar Set	ASTM A615
Units	English
Run Option	Investigation
Run Axis	Biaxial
Slenderness	Not Considered
Column Type	Structural
Capacity Method	Critical capacity

Materials

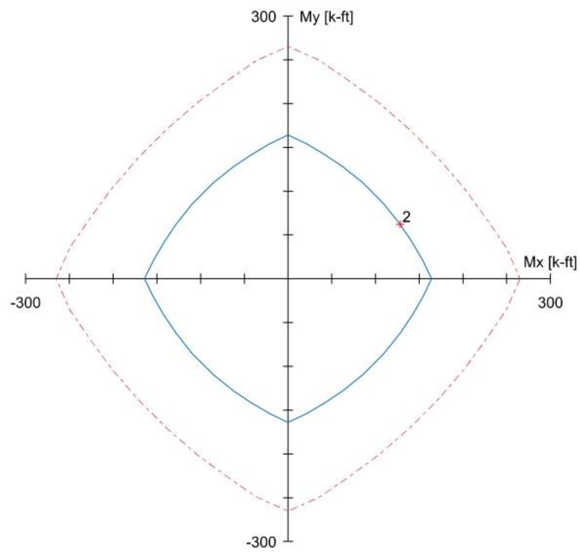
f'_c	4 ksi
E_c	3605 ksi
f_y	60 ksi
E_s	29000 ksi

Section

Type	Irregular
A_g	256 in²
I_x	5461.33 in⁴
I_y	5461.33 in⁴

Reinforcement

Pattern	Irregular
Bar layout	---
Cover to	---
Clear cover	---
Bars	---
Confinement type	Tied
Total steel area, A_s	6.32 in²
Rho	2.47 %
Min. clear spacing	4.60 in



No.	P_u	M_{ux}	M_{uy}	ϕP_n	ϕM_{nx}	ϕM_{ny}	Capacity
	kip	k-ft	k-ft	kip	k-ft	k-ft	Ratio
2	314.7	128.1	62.2	314.68	128.15	62.19	1.00

Max. Capacity Ratio: 1.54

3. Summary and Comparison of Design Results

Table 2 - Comparison of Results			
Parameter	Reference	Hand	spColumn
c, in.	12.66	12.66	12.64
d_5, in.	18.58	18.58	18.58
ε_{s5}, in./in.	0.00140	0.00140	0.00141
ϕP_n, kip	315.0	315.60	314.68
ϕM_{nx}, kip-ft	128.33	128.12	128.15
ϕM_{ny}, kip-ft	62.33	62.12	62.19

In all of the hand calculations and the reference used illustrated above, the results are in good agreement with the automated exact results obtained from the spColumn program.

4. Conclusions & Observations

The analysis of the reinforced concrete section performed by spColumn conforms to the provisions of the Strength Design Method and Unified Design Provisions with all conditions of strength satisfying the applicable conditions of equilibrium and strain compatibility.

In most building design calculations, such as the examples shown for flat plate or flat slab concrete floor systems, all building columns may be subjected to biaxial bending (M_x and M_y) due to lateral effects and unbalanced moments from both directions of analysis. This requires an investigation of the column P-M_x-M_y interaction diagram in two directions simultaneously (axial force interaction with biaxial bending).

This example shows the calculations needed to obtain one point on the three-dimensional failure surface (biaxial M_x-M_y interaction diagram). Generating the three-dimensional failure surface (interaction diagram) for a column section subjected to a combined axial force and biaxial bending moments is tedious and challenging for engineers and the use of a computer aid can save time and eliminate errors. StucturePoint's spColumn program can, quickly, simply and accurately generate the three-dimensional failure surface (interaction diagram) for all commonly encountered column, beam or wall sections in addition to highly complex and irregular cross-sections.

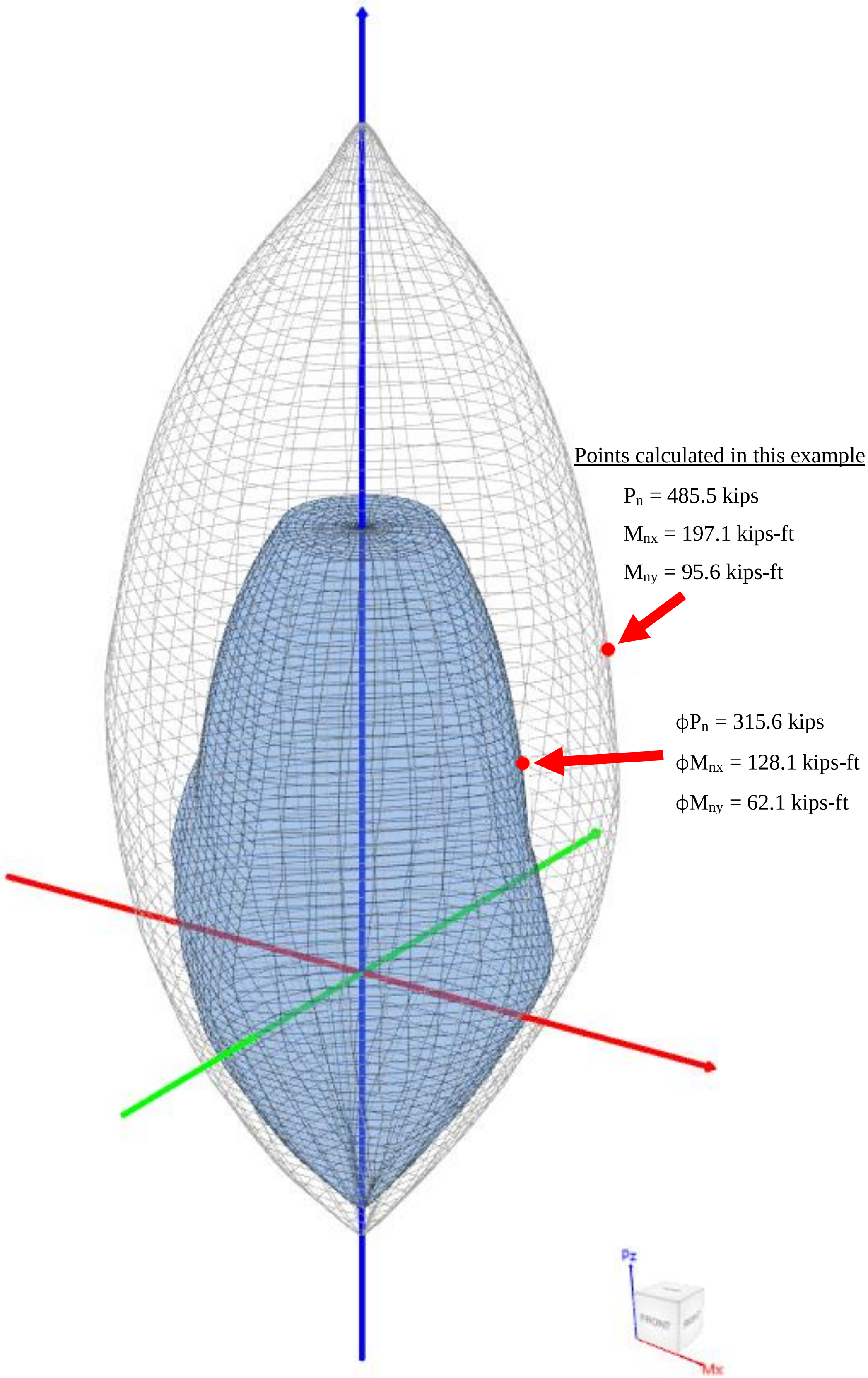


Figure 10 – Interaction Diagram in Two Directions (Biaxial) (spColumn)

The spColumn "Diagrams" module is a powerful tool especially for investigating interaction diagrams (failure surfaces) for columns and walls sections subjected to a combined axial force and biaxial bending moments. The module allows the user to view and analyze 2D interaction diagrams and contours along with 3D failure surfaces in a multi viewport environment. The following figure shows three views of:

1. P-M interaction diagram cut at angle of 26º
2. M_x-M_y interaction diagram cut at axial load of 314.7 kip in compression
3. A 3D failure surface (interaction diagram) showing the points calculated in this example.

Figures 12, 13, and 14 show 3D visualization of failure surface with a horizontal and vertical plane cut, respectively.

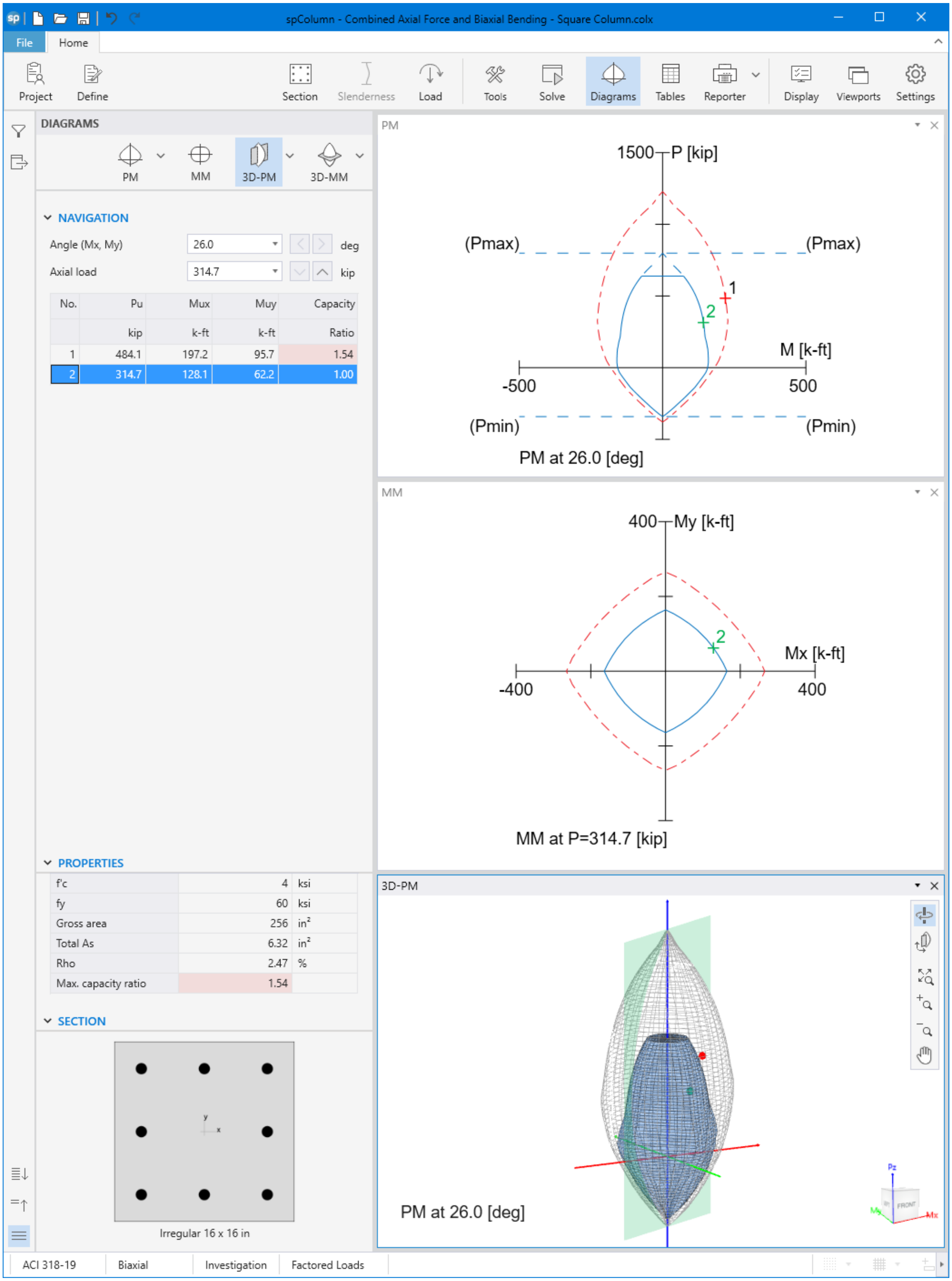


Figure 11 – Diagrams Module (spColumn)

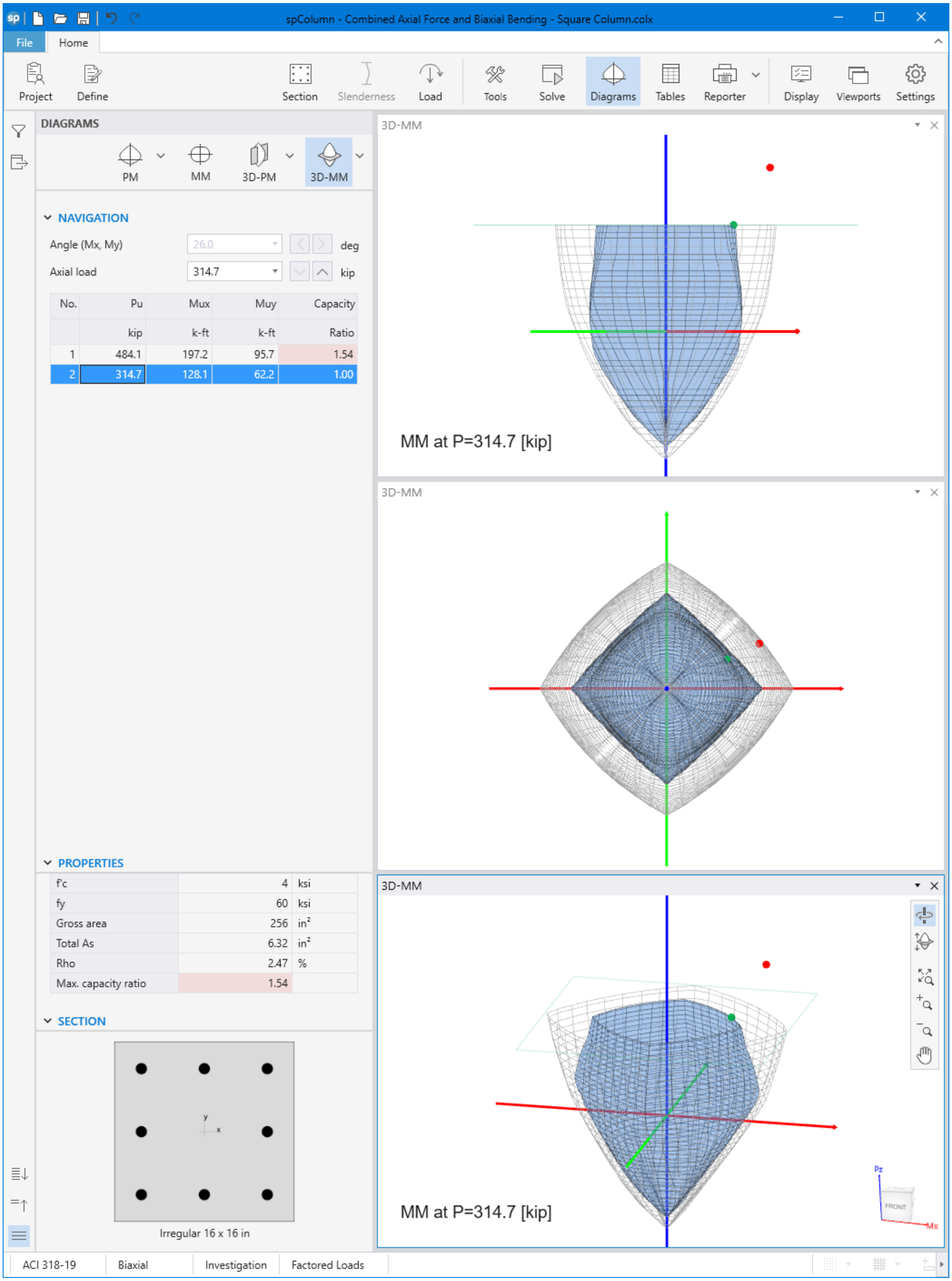


Figure 12 – 3D Visualization of Failure Surface with a Horizontal Plane Cut at P = 314.7 kip (spColumn)

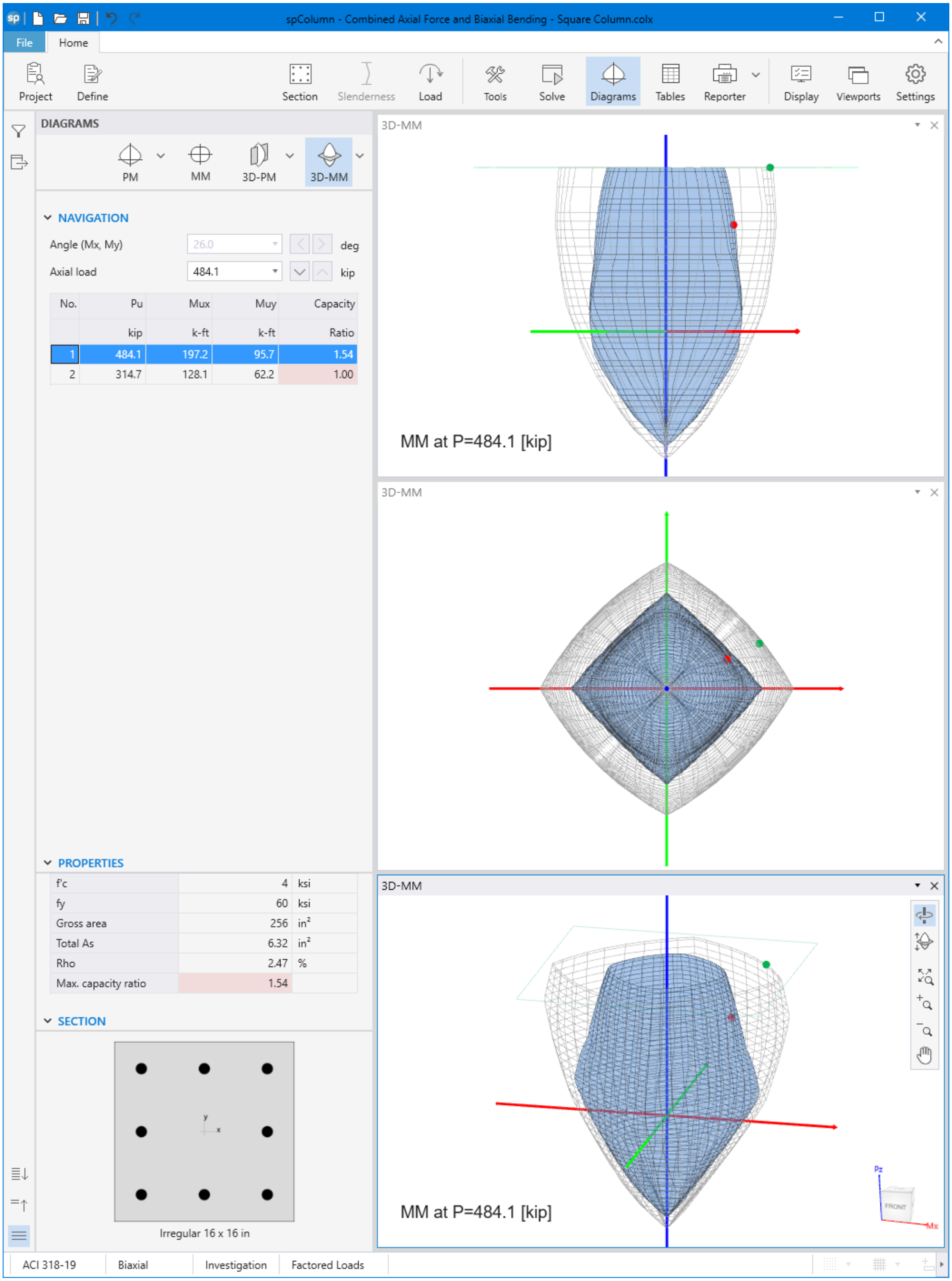


Figure 13 – 3D Visualization of Failure Surface with a Horizontal Plane Cut at P = 484.1 kip (spColumn)

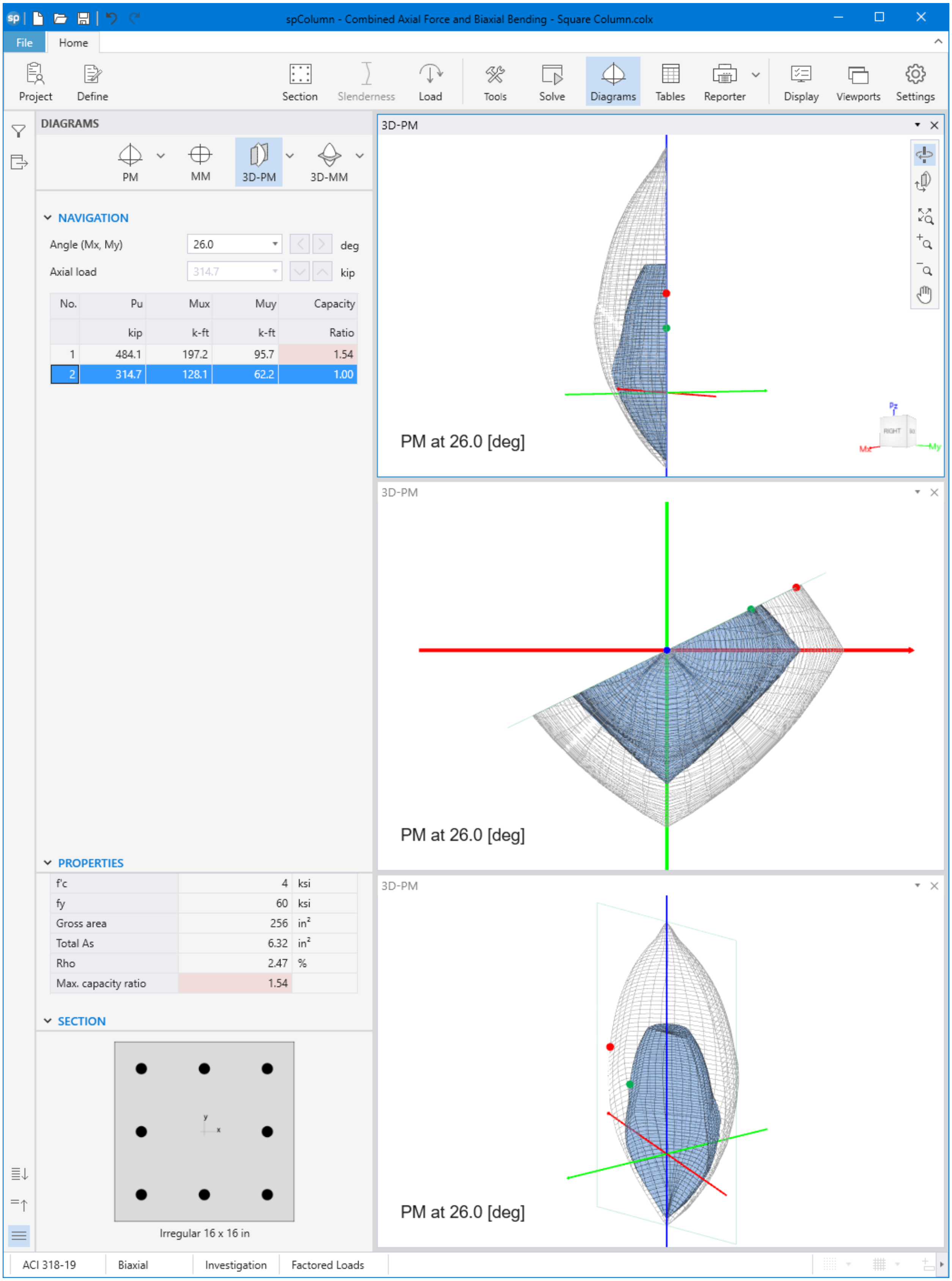


Figure 14 – 3D Visualization of Failure Surface with a Vertical Plane Cut at 26º (spColumn)