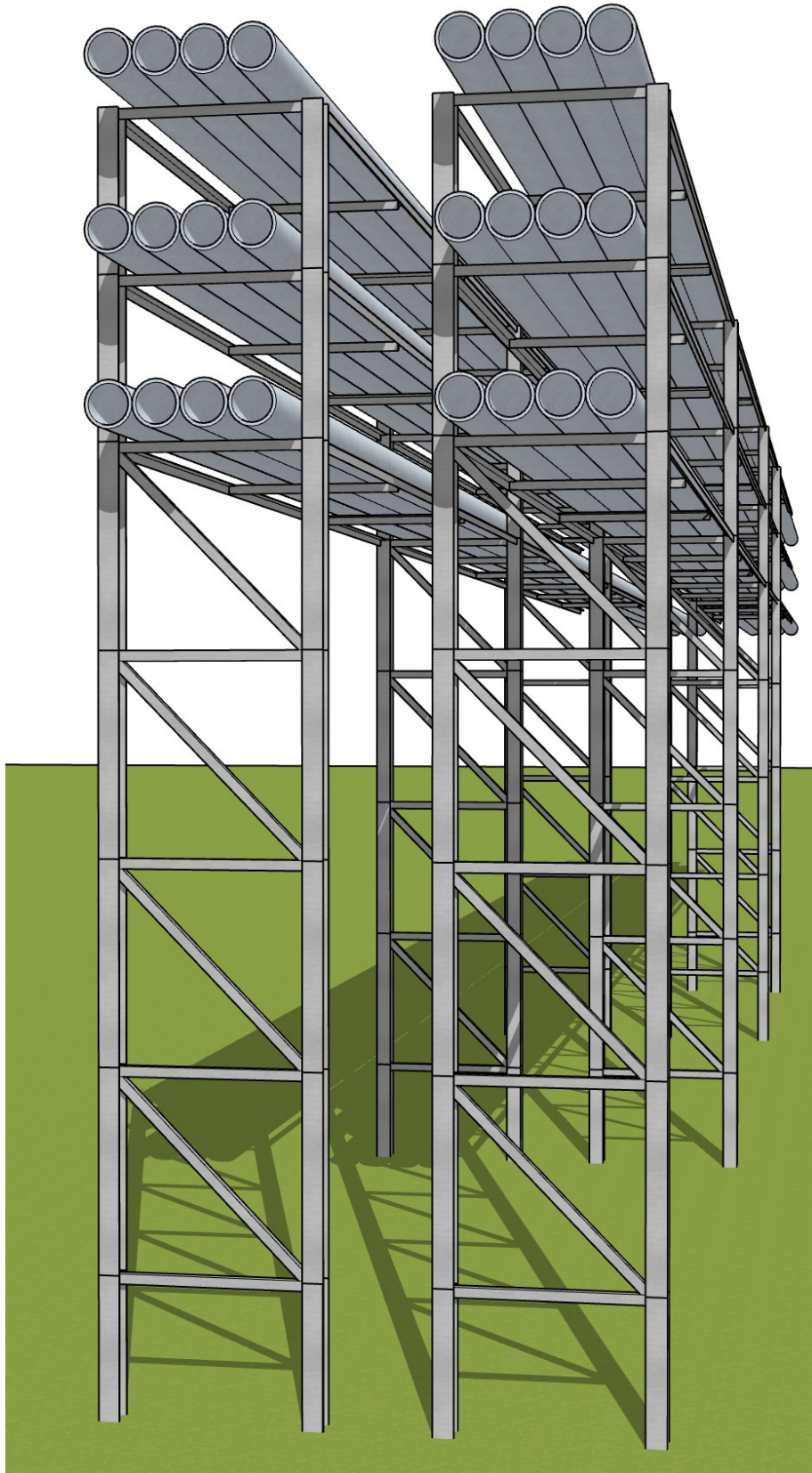
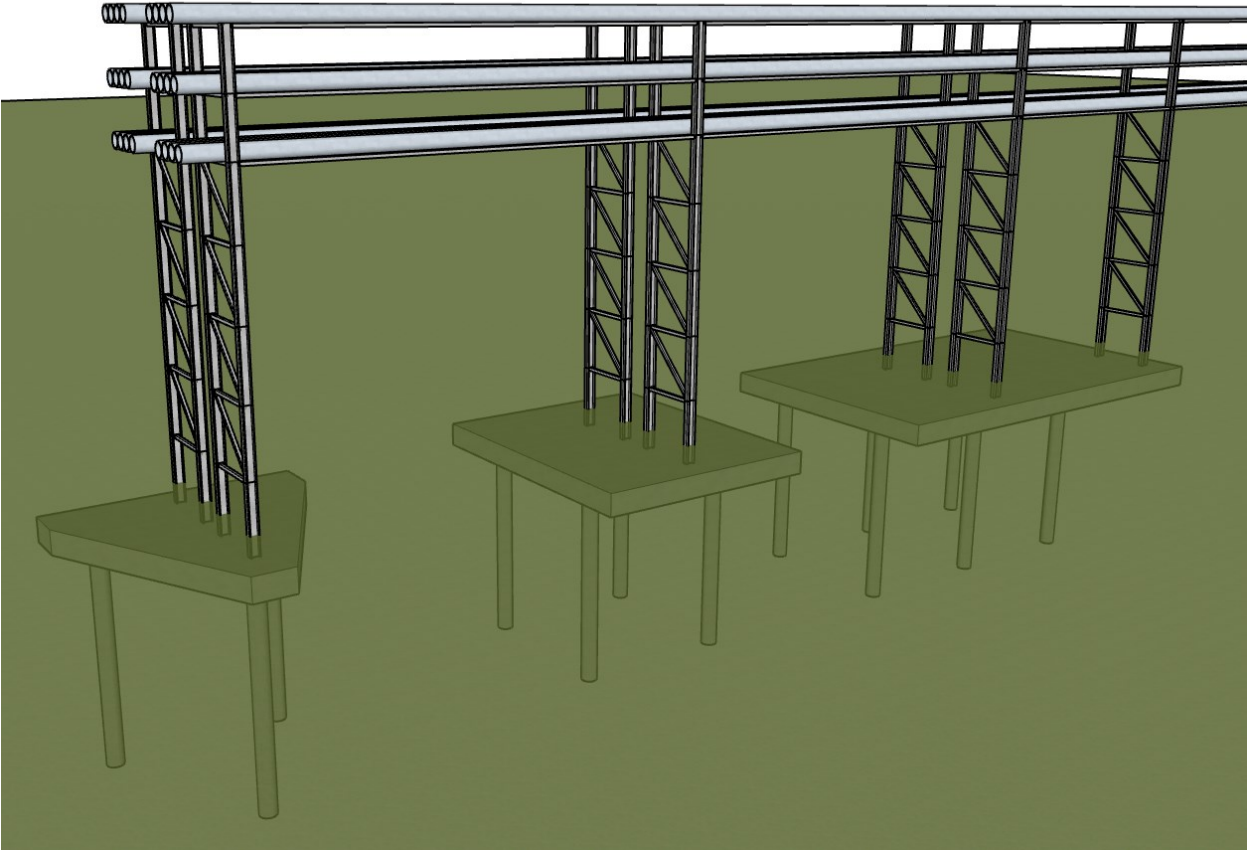
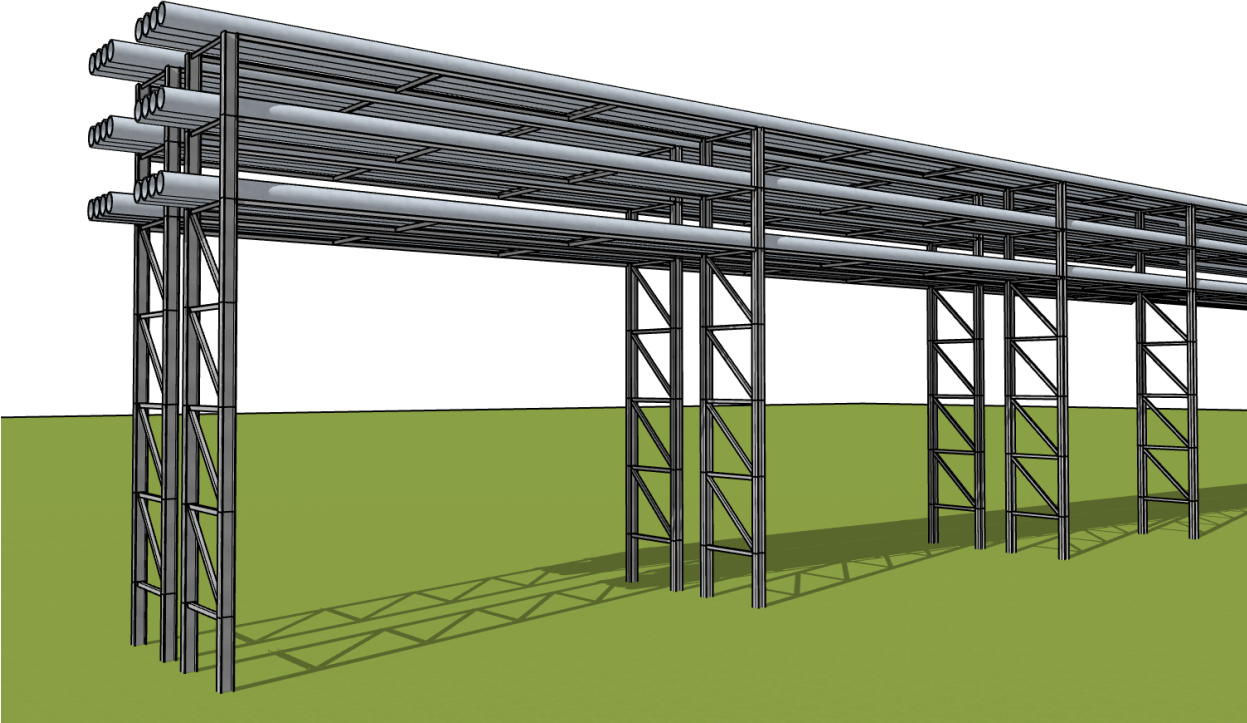


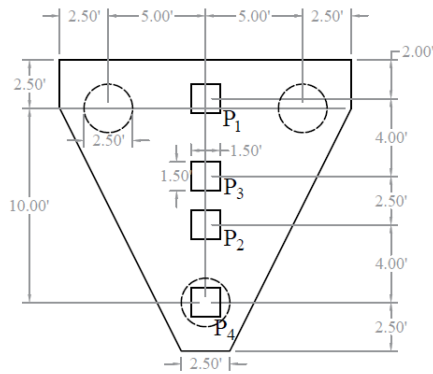
Industrial Plant Pipe Rack Foundations Analysis and Design



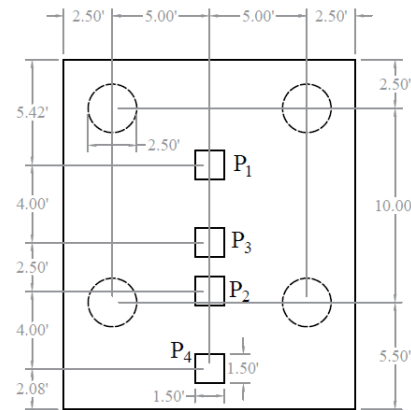


Industrial Plant Pipe Rack Foundations Analysis and Design

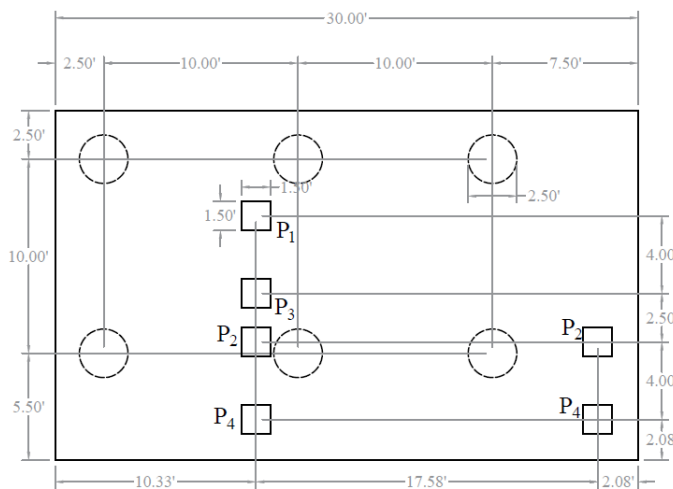
Industrial pipe racks typically support pipes, power cables and instrument cable trays in petrochemical, chemical, paper mills and power plants. Occasionally, pipe racks may also support mechanical equipment, vessels and valve access platforms. Main pipe racks generally transfer process material between equipment and storage or utility areas where operation specific load cases and temperature effect often complicates the structural analysis and the resulting foundation reactions. Storage racks found in warehouses are not pipe racks, even if they store lengths of pipe. This case study focuses on the design of pipe rack foundations using the engineering software program [spMats](#). All the information provided by the structural engineer regarding the pipe rack foundations are shown in the following figure and design data section and will serve as input for foundation design. Because of poor soil conditions at the site and tower height, significant uplift is expected and a pile supported foundation is selected to resist the design overturning moments. 30" diameter piles are assembled in pile caps as shown in the following figures.



Pile Cap 1



Pile Cap 2



Pile Cap 3

Figure 1 – Industrial Plant Pile Cap Foundations Types

Code

Building Code Requirements for Structural Concrete (ACI 318-14) and Commentary (ACI 318R-14)

Reference

spMats Engineering Software Program Manual v8.50, StructurePoint LLC., 2016

Design Data

Concrete Piers

Size = 1.5 ft x 1.5 ft

Pile Cap Foundations

$f_c' = 3,000$ psi

$f_y = 60,000$ psi

Thickness = 3 ft

Clear Cover = 2 in.

Concrete Piles

$f_c' = 4,000$ psi

$f_y = 60,000$ psi

Diameter = 2.5 ft

Clear Cover = 3 in.

Length = 33 ft

Pile embedment = 6 in.

Foundation Loads

Load Case	Load*, kips			
	P ₁	P ₂	P ₃	P ₄
Dead	-6**	65	165	4
Live	251	35	85	-45**
Wind	40	-160**	-35**	160

* Load locations are shown in the following figure

** Negative values indicate uplift loads

Contents

1. Foundation Analysis and Design – spMats Software	1
2. Two-way Punching Shear Check	11
3. Pile Reactions	13
4. Pile Cap Model Statistics	15
5. Column and Pile Design - spColumn	16
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7. Tied vs. Spiral Confinement.....	21

1. Foundation Analysis and Design – spMats Software

[spMats](#) uses the Finite Element Method for the structural modeling, analysis and design of reinforced concrete slab systems or mat foundations subject to static loading conditions.

The slab, mat, or footing is idealized as a mesh of rectangular elements interconnected at the corner nodes. The same mesh applies to the underlying soil with the soil stiffness concentrated at the nodes. Slabs of irregular geometry can be idealized to conform to geometry with rectangular boundaries. Even though slab and soil properties can vary between elements, they are assumed uniform within each element. Piles are modeled as springs connected to the nodes of the finite element model. Unlike for springs, however, punching shear check is performed around piles.

For illustration and purposes, the following figures provide a sample of the input modules and results obtained from spMats models created for the industrial plant pipe rack foundations (pile caps) in this example.

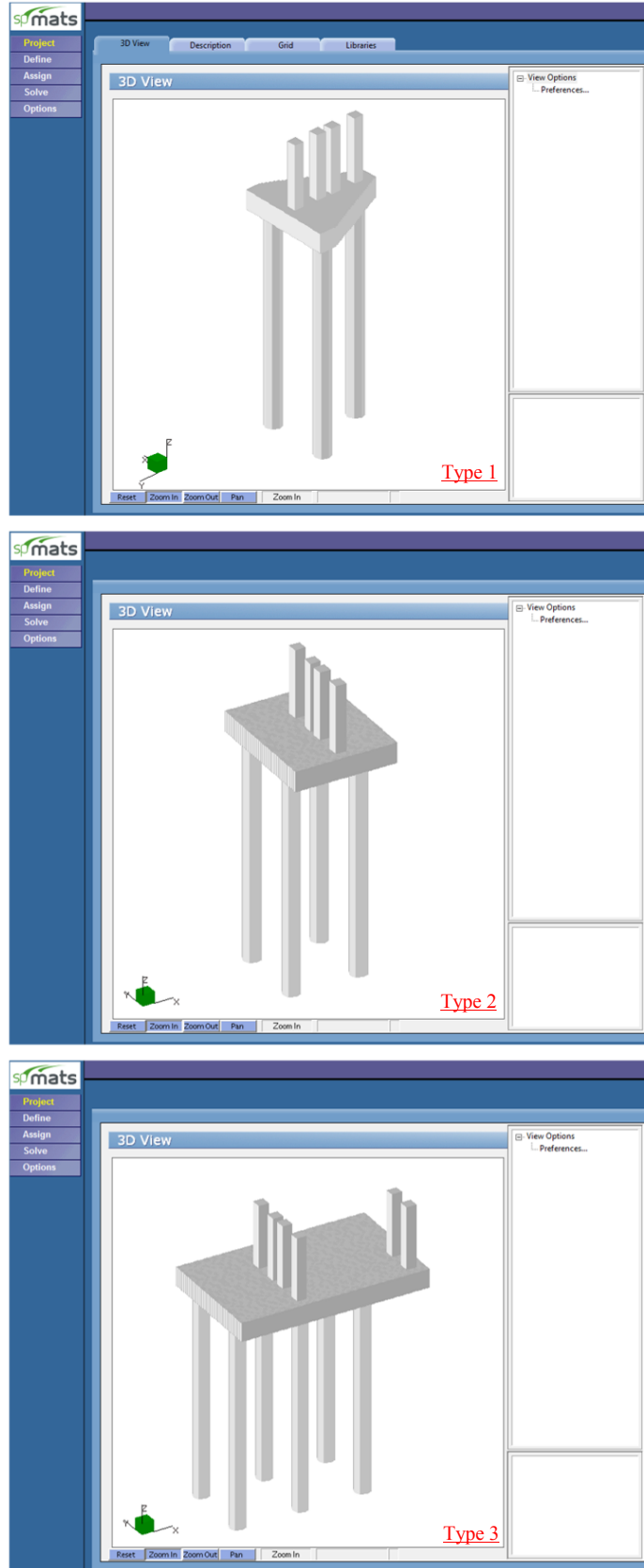


Figure 2 – Pipe Rack Foundation Model - 3D Views

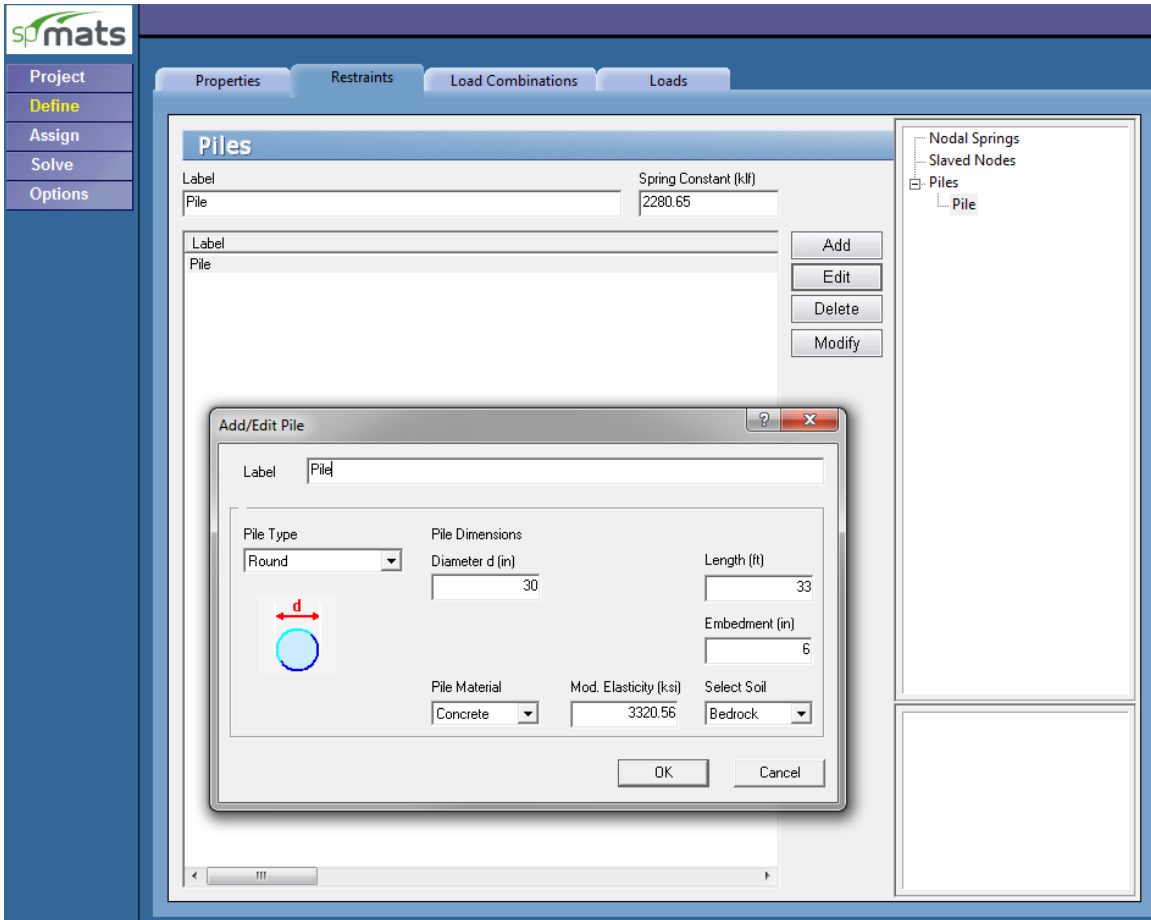


Figure 3 –Defining Piles

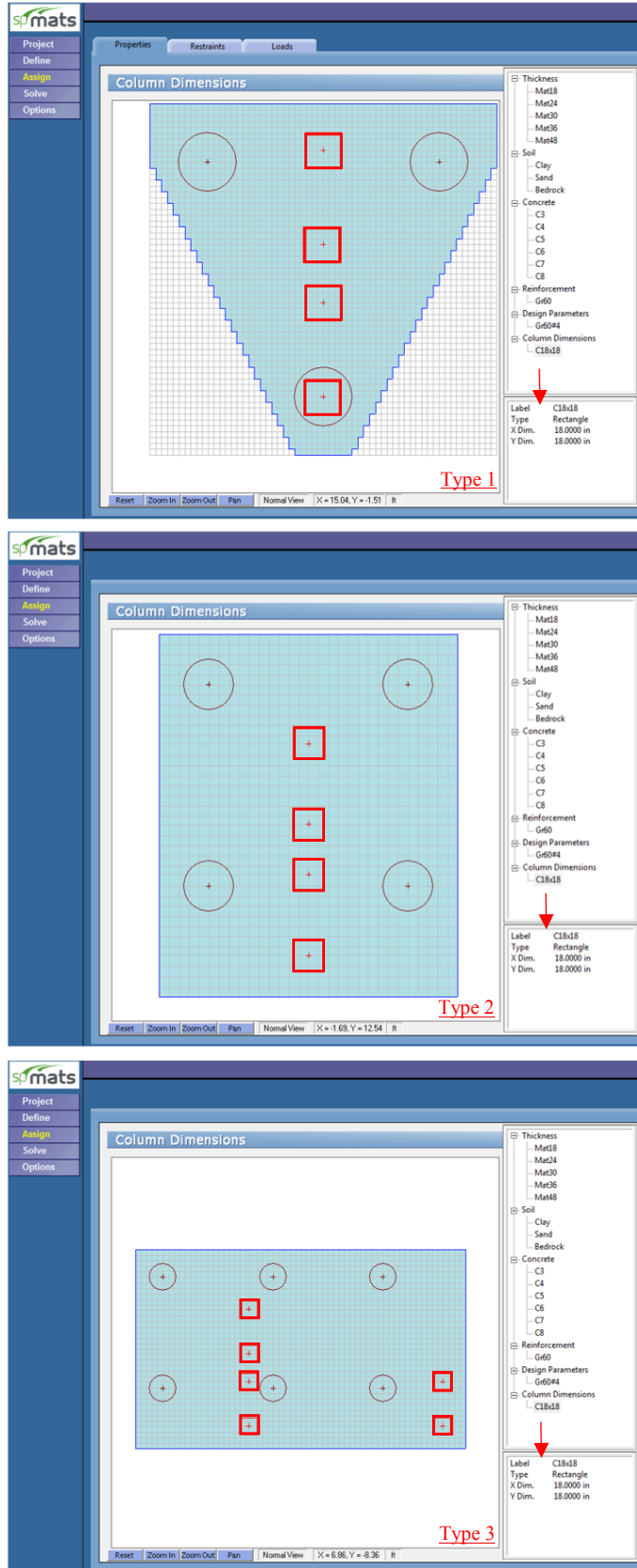


Figure 4 – Assigning Concrete Piers

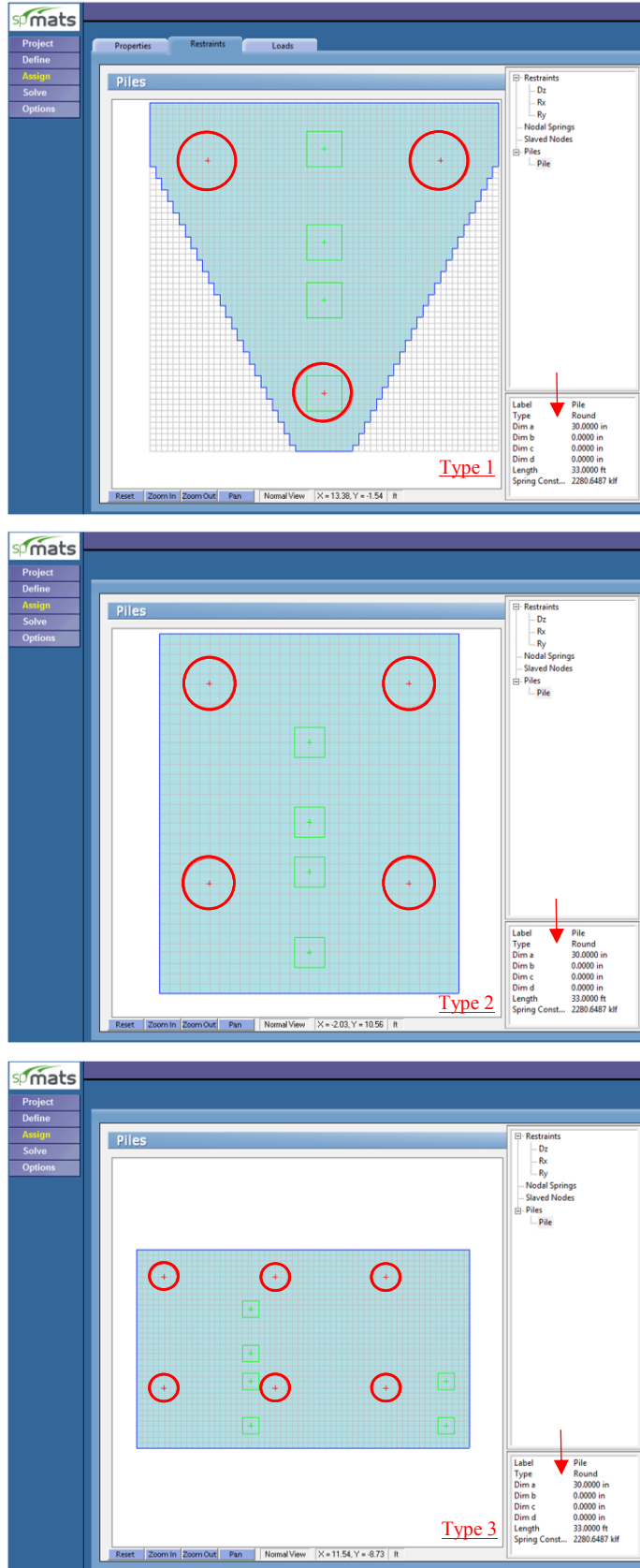


Figure 5 – Assigning Piles

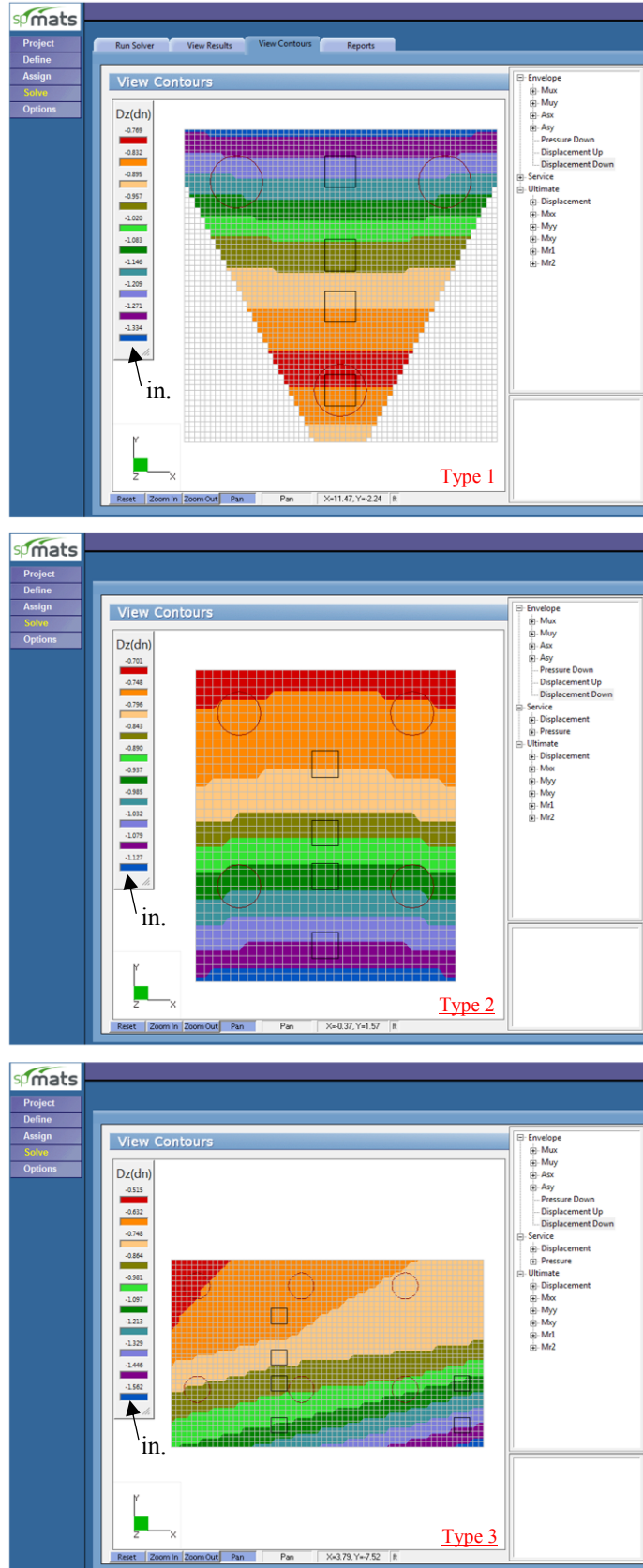


Figure 6 – Vertical Displacement Contours (Note cantilevered corner displacement)

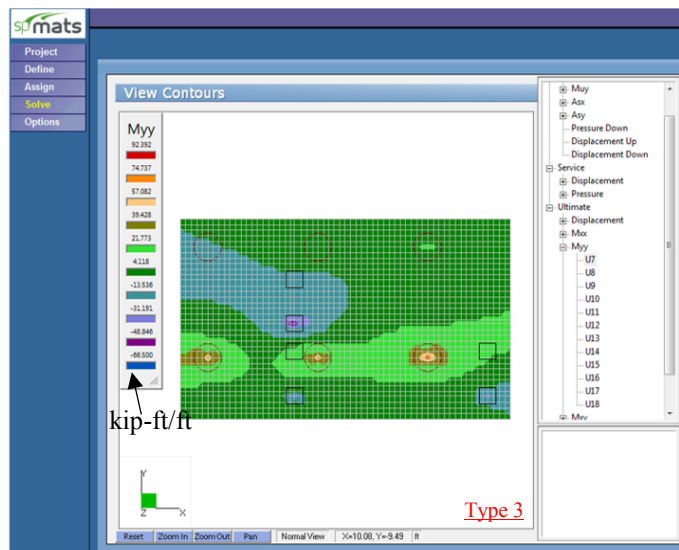
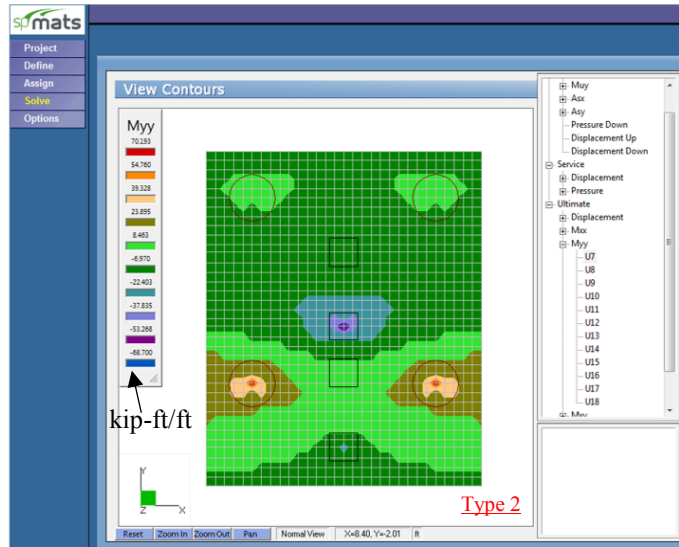
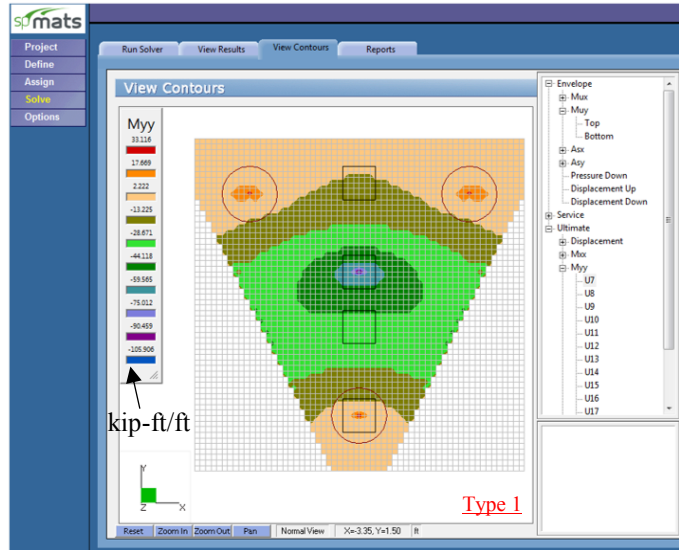


Figure 7 –Moment Contours along Y-Axis

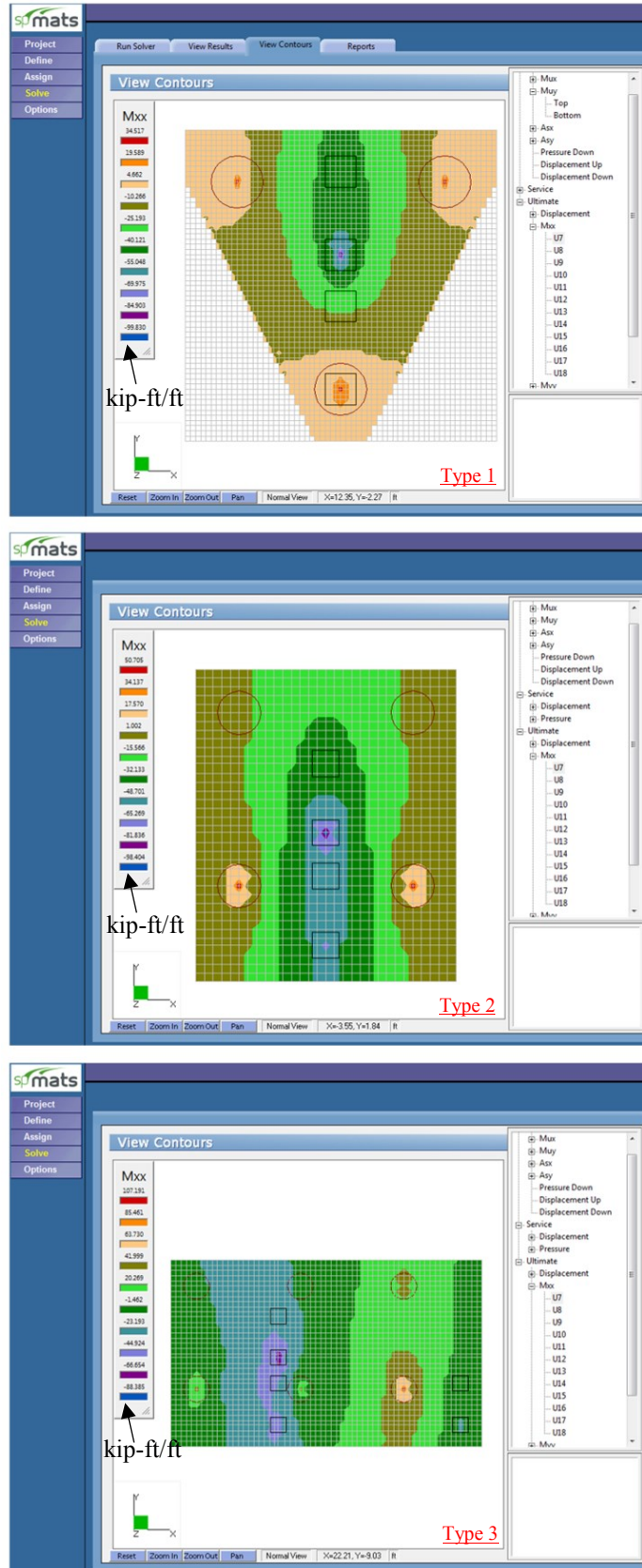


Figure 8 – Moment Contours along X-Axis - Complete Model

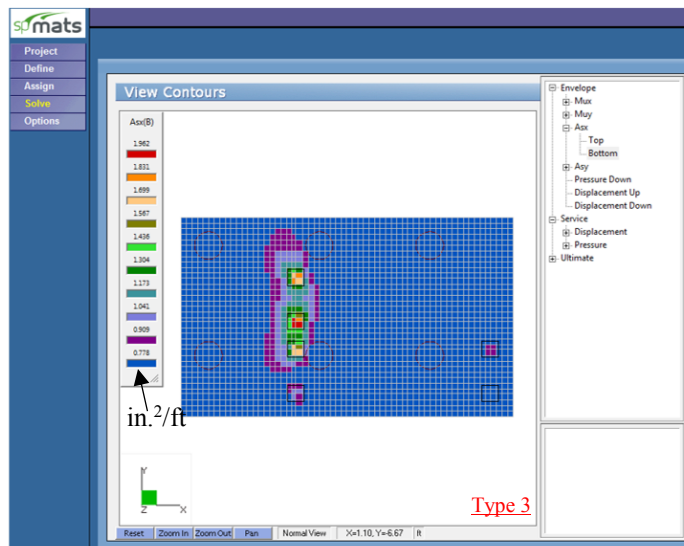
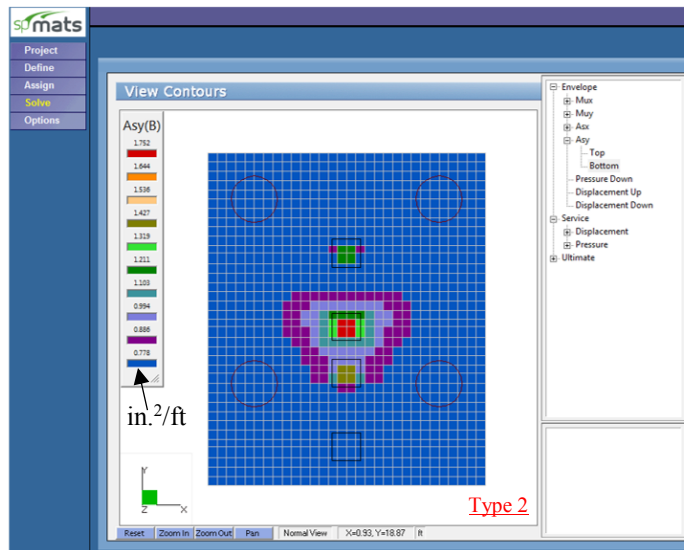
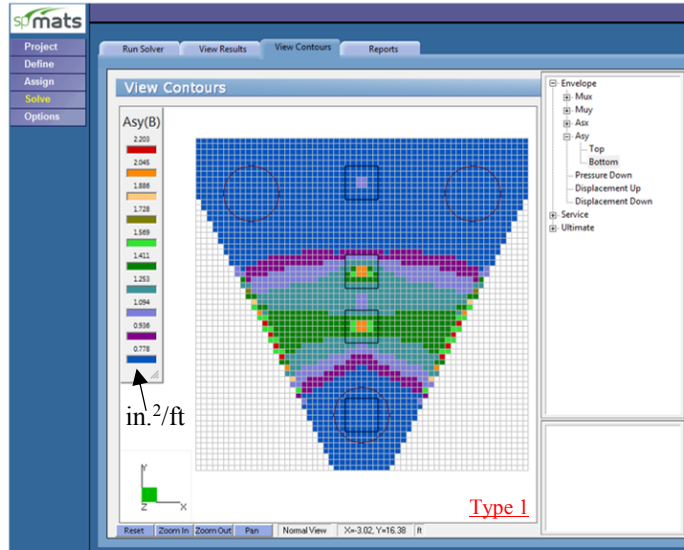


Figure 9 – Required Reinforcement Contours along Y Direction

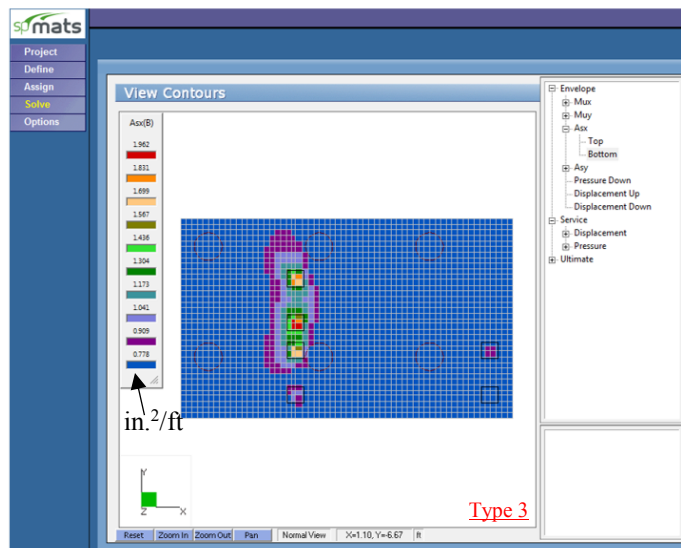
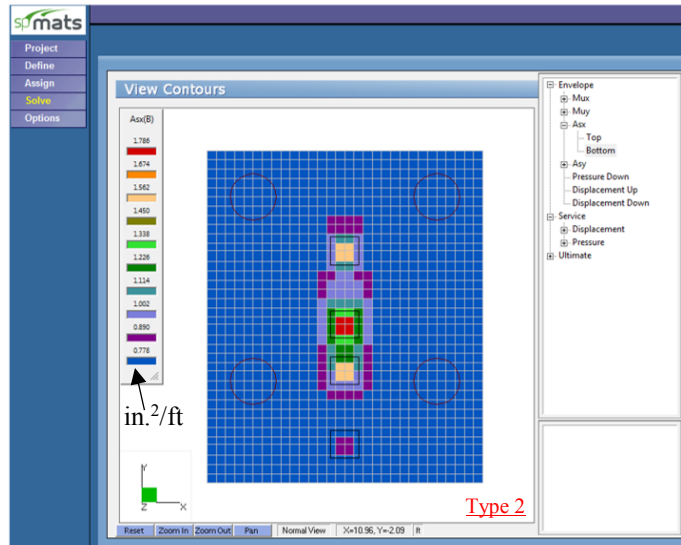
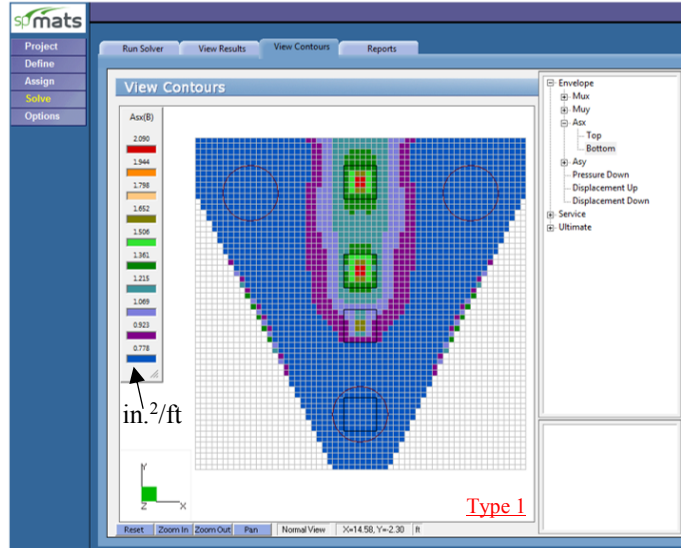


Figure 10 – Required Reinforcement Contours along X Direction

2. Two-way Punching Shear Check

According to ACI 312-14 (R13.2.7.2), if shear perimeters overlap, the modified critical perimeter should be taken as that portion of the smallest envelope of individual shear perimeters that will actually resist the critical shear for group under consideration. [spMats](#) reports standard shear perimeter for three conditions (interior, edge, and corner) only considering adequate spacing and edge distance is provided to prevent overlapping or truncated shear perimeter.

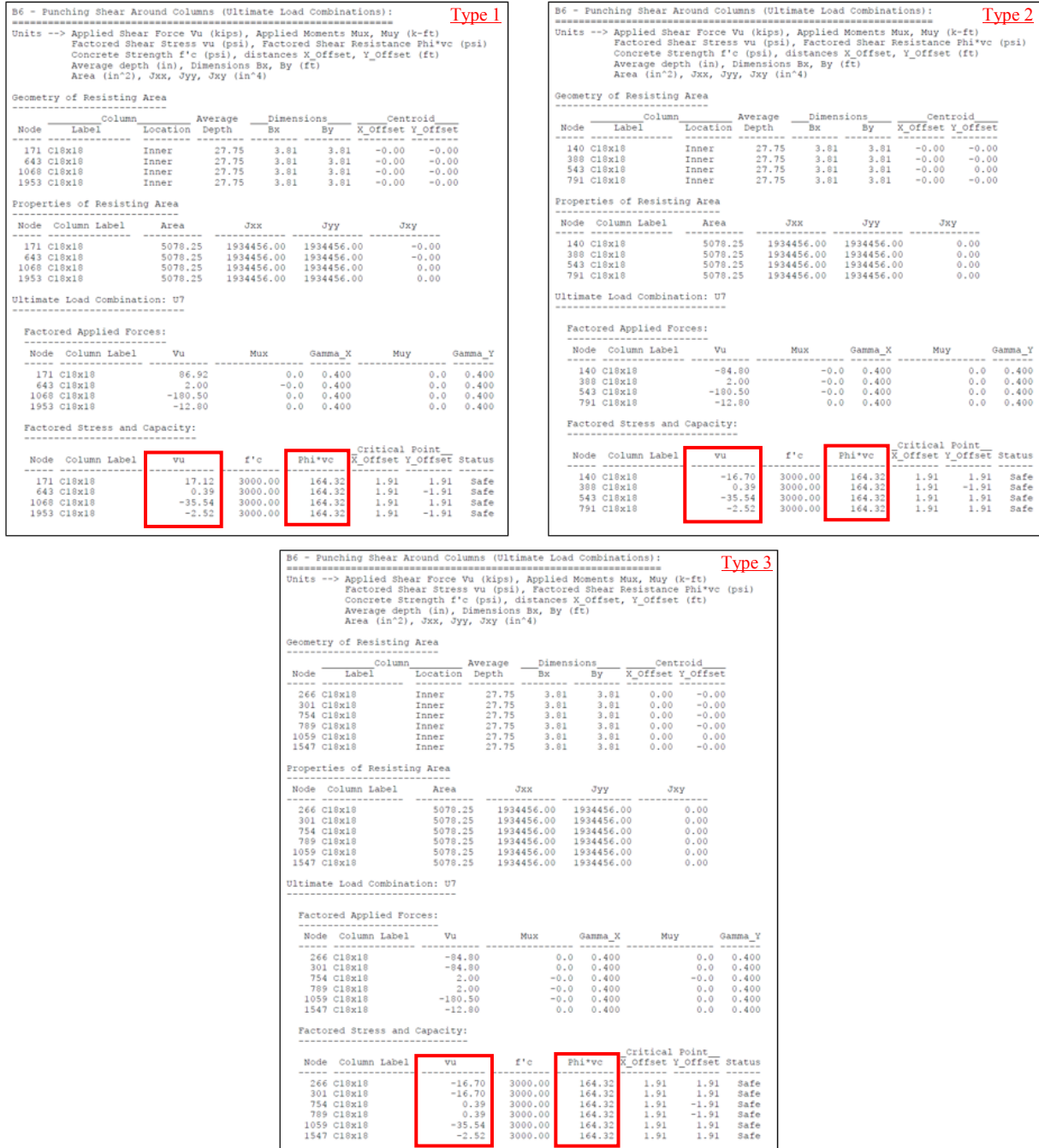


Figure 11 – Two-Way Shear Results around Concrete Piers

B7 - Punching Shear Around Piles (Ultimate Load Combinations): Type 1

Units --> Applied Shear Force Vu (kips), Applied Moments Mux, Muy (k-ft)
Factored Shear Stress vu (psi), Factored Shear Resistance vc (psi)
Concrete Strength f'c (psi), distances X_Offset, Y_Offset (ft)
Average depth (in), Dimensions Bx, By (ft)
Area (in²), Jxx, Jyy, Jxy (in⁴)

Geometry of Resisting Area

Node	File Label	Location	Average Depth	Dimensions Bx	By	X_Offset	Y_Offset
171 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
1811 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
1851 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00

Properties of Resisting Area

Node	File Label	Area	Jxx	Jyy	Jxy
171 Pile		5029.43	2249450.50	2249450.00	0.00
1811 Pile		5029.43	2249450.25	2249450.00	0.00
1851 Pile		5029.42	2249450.00	2249450.00	0.00

Ultimate Load Combination: U7

Factored Applied Forces:

Node	File Label	Vu	Mux	Gamma_X	Muy	Gamma_Y
171 Pile		86.92	-0.0	0.400	0.0	0.400
1811 Pile		91.85	0.0	0.400	0.0	0.400
1851 Pile		91.85	0.0	0.400	-0.0	0.400

Factored Stress and Capacity:

Node	File Label	vu	f'c	Phi*vc	Critical Point X_Offset	Y_Offset	Status
171 Pile		17.28	3000.00	164.32	-2.38	-0.38	Safe
1811 Pile		18.26	3000.00	164.32	-2.41	0.00	Safe
1851 Pile		18.26	3000.00	164.32	1.41	1.95	Safe

B7 - Punching Shear Around Piles (Ultimate Load Combinations): Type 2

Units --> Applied Shear Force Vu (kips), Applied Moments Mux, Muy (k-ft)
Factored Shear Stress vu (psi), Factored Shear Resistance vc (psi)
Concrete Strength f'c (psi), distances X_Offset, Y_Offset (ft)
Average depth (in), Dimensions Bx, By (ft)
Area (in²), Jxx, Jyy, Jxy (in⁴)

Geometry of Resisting Area

Node	File Label	Location	Average Depth	Dimensions Bx	By	X_Offset	Y_Offset
347 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
367 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
967 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
987 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00

Properties of Resisting Area

Node	File Label	Area	Jxx	Jyy	Jxy
347 Pile		5029.42	2249450.25	2249449.75	0.00
367 Pile		5029.42	2249450.00	2249450.00	0.00
967 Pile		5029.43	2249450.25	2249450.00	0.00
987 Pile		5029.42	2249450.00	2249450.00	0.00

Ultimate Load Combination: U7

Factored Applied Forces:

Node	File Label	Vu	Mux	Gamma_X	Muy	Gamma_Y
347 Pile		167.67	-0.0	0.400	0.0	0.400
367 Pile		167.67	-0.0	0.400	-0.0	0.400
967 Pile		43.28	0.0	0.400	0.0	0.400
987 Pile		43.28	0.0	0.400	-0.0	0.400

Factored Stress and Capacity:

Node	File Label	vu	f'c	Phi*vc	Critical Point X_Offset	Y_Offset	Status
347 Pile		33.34	3000.00	164.32	-0.74	-2.29	Safe
367 Pile		33.34	3000.00	164.32	0.00	-2.41	Safe
967 Pile		8.61	3000.00	164.32	-2.41	-0.00	Safe
987 Pile		8.61	3000.00	164.32	1.95	1.41	Safe

B7 - Punching Shear Around Piles (Ultimate Load Combinations): Type 3

Units --> Applied Shear Force Vu (kips), Applied Moments Mux, Muy (k-ft)
Factored Shear Stress vu (psi), Factored Shear Resistance vc (psi)
Concrete Strength f'c (psi), distances X_Offset, Y_Offset (ft)
Average depth (in), Dimensions Bx, By (ft)
Area (in²), Jxx, Jyy, Jxy (in⁴)

Geometry of Resisting Area

Node	File Label	Location	Average Depth	Dimensions Bx	By	X_Offset	Y_Offset
677 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
697 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
717 Pile	Inner		27.75	4.81	4.81	0.00	-0.00
1897 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
1917 Pile	Inner		27.75	4.81	4.81	-0.00	-0.00
1937 Pile	Inner		27.75	4.81	4.81	0.00	-0.00

Properties of Resisting Area

Node	File Label	Area	Jxx	Jyy	Jxy
677 Pile		5029.42	2249450.25	2249449.75	0.00
697 Pile		5029.42	2249450.00	2249450.00	0.00
717 Pile		5029.42	2249449.50	2249449.50	0.00
1897 Pile		5029.43	2249450.25	2249450.00	0.00
1917 Pile		5029.42	2249450.00	2249450.00	0.00
1937 Pile		5029.42	2249449.50	2249449.75	0.00

Ultimate Load Combination: U7

Factored Applied Forces:

Node	File Label	Vu	Mux	Gamma_X	Muy	Gamma_Y
677 Pile		143.97	-0.0	0.400	0.0	0.400
697 Pile		181.38	-0.0	0.400	-0.0	0.400
717 Pile		216.82	-0.0	0.400	-0.0	0.400
1897 Pile		1.85	0.0	0.400	0.0	0.400
1917 Pile		36.74	0.0	0.400	-0.0	0.400
1937 Pile		69.94	0.0	0.400	-0.0	0.400

Factored Stress and Capacity:

Node	File Label	vu	f'c	Phi*vc	Critical Point X_Offset	Y_Offset	Status
677 Pile		28.62	3000.00	164.32	-0.74	-2.29	Safe
697 Pile		36.06	3000.00	164.32	0.00	-2.41	Safe
717 Pile		43.07	3000.00	164.32	2.38	-0.38	Safe
1897 Pile		0.37	3000.00	164.32	-2.41	-0.00	Safe
1917 Pile		7.30	3000.00	164.32	1.41	1.95	Safe
1937 Pile		13.91	3000.00	164.32	2.41	-0.00	Safe

Figure 12 – Two-Way Shear Results around Piles

3. Pile Reactions

The model results provide a detailed list of the pile reactions indicating the magnitude and direction of the resulting forces on each pile in the foundation model. Whether force is downward compression or upward net tension on the pile, the load combination producing the maximum reaction is denoted in the output results table.

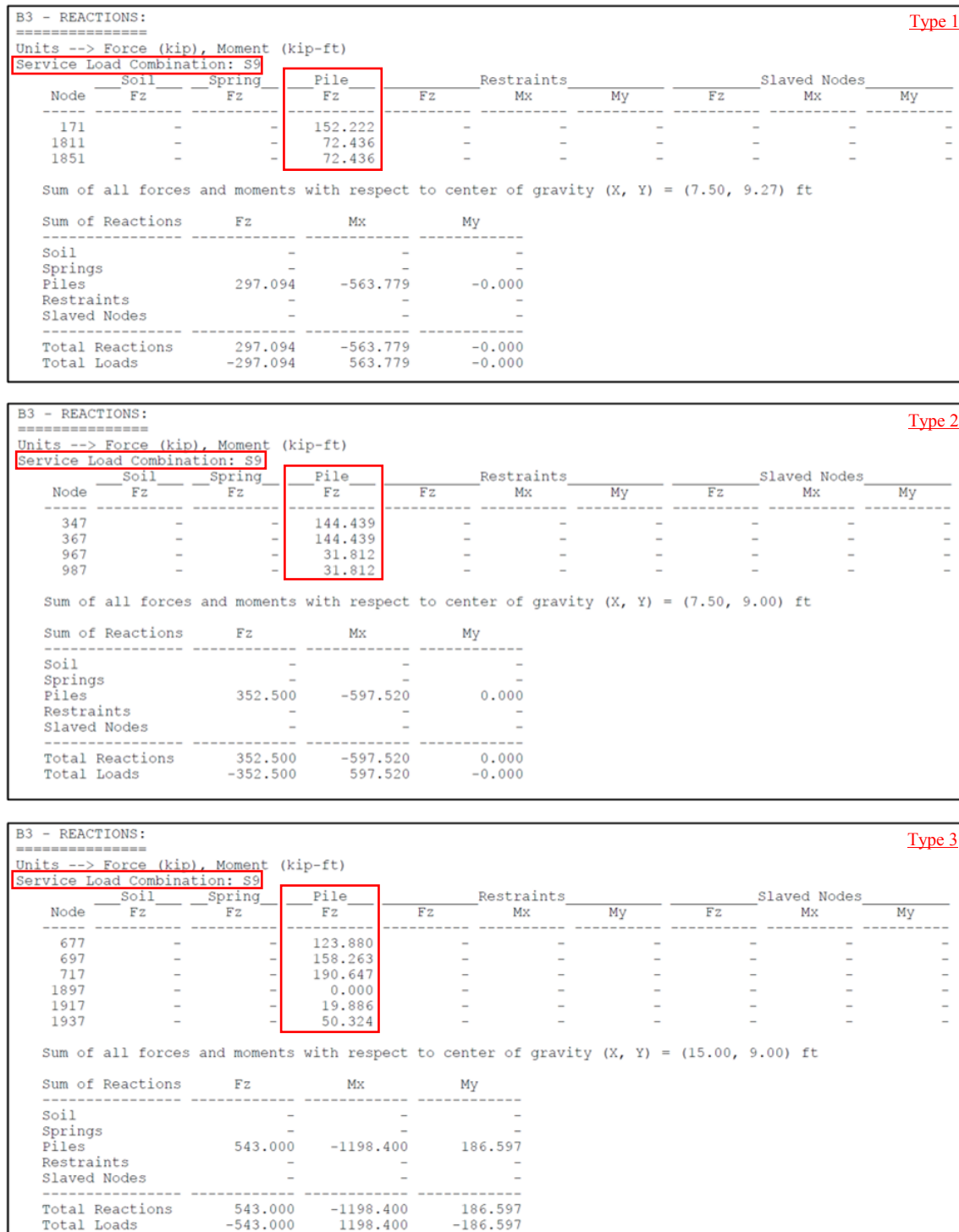


Figure 13 – Piles Service Reactions

Type 1

```

B3 - REACTIONS:
=====
Units --> Force (kip), Moment (kip-ft)
Ultimate Load Combination: U7
    
```

Node	Soil	Spring	Pile	Restraints			Slaved Nodes		
	Fz	Fz	Fz	Fz	Mx	My	Fz	Mx	My
171	-	-	171.722	-	-	-	-	-	-
1811	-	-	91.845	-	-	-	-	-	-
1851	-	-	91.845	-	-	-	-	-	-

Sum of all forces and moments with respect to center of gravity (X, Y) = (7.50, 9.27) ft

Sum of Reactions	Fz	Mx	My
Soil	-	-	-
Springs	-	-	-
Piles	355.413	-570.634	-0.000
Restraints	-	-	-
Slaved Nodes	-	-	-
Total Reactions	355.413	-570.634	-0.000
Total Loads	-355.413	570.634	-0.000

Type 2

```

B3 - REACTIONS:
=====
Units --> Force (kip), Moment (kip-ft)
Ultimate Load Combination: U7
    
```

Node	Soil	Spring	Pile	Restraints			Slaved Nodes		
	Fz	Fz	Fz	Fz	Mx	My	Fz	Mx	My
347	-	-	167.666	-	-	-	-	-	-
367	-	-	167.666	-	-	-	-	-	-
967	-	-	43.284	-	-	-	-	-	-
987	-	-	43.284	-	-	-	-	-	-

Sum of all forces and moments with respect to center of gravity (X, Y) = (7.50, 9.00) ft

Sum of Reactions	Fz	Mx	My
Soil	-	-	-
Springs	-	-	-
Piles	421.900	-610.962	0.000
Restraints	-	-	-
Slaved Nodes	-	-	-
Total Reactions	421.900	-610.962	0.000
Total Loads	-421.900	610.962	-0.000

Type 3

```

B3 - REACTIONS:
=====
Units --> Force (kip), Moment (kip-ft)
Ultimate Load Combination: U7
    
```

Node	Soil	Spring	Pile	Restraints			Slaved Nodes		
	Fz	Fz	Fz	Fz	Mx	My	Fz	Mx	My
677	-	-	143.966	-	-	-	-	-	-
697	-	-	181.383	-	-	-	-	-	-
717	-	-	216.620	-	-	-	-	-	-
1897	-	-	1.851	-	-	-	-	-	-
1917	-	-	36.737	-	-	-	-	-	-
1937	-	-	69.944	-	-	-	-	-	-

Sum of all forces and moments with respect to center of gravity (X, Y) = (15.00, 9.00) ft

Sum of Reactions	Fz	Mx	My
Soil	-	-	-
Springs	-	-	-
Piles	650.500	-1191.429	218.783
Restraints	-	-	-
Slaved Nodes	-	-	-
Total Reactions	650.500	-1191.429	218.783
Total Loads	-650.500	1191.429	-218.783

Figure 14 – Piles Ultimate Reactions

Note: Positive and negative reaction values indicate compression and tension forces in piles, respectively.

4. Pile Cap Model Statistics

Since spMats is utilizing finite element analysis to model and design the foundation. It is useful to track the number of elements and nodes used in the model to optimize the model results (accuracy) and running time (processing stage). spMats provides model statistics to keep tracking the mesh sizing as a function of the number of nodes and elements.

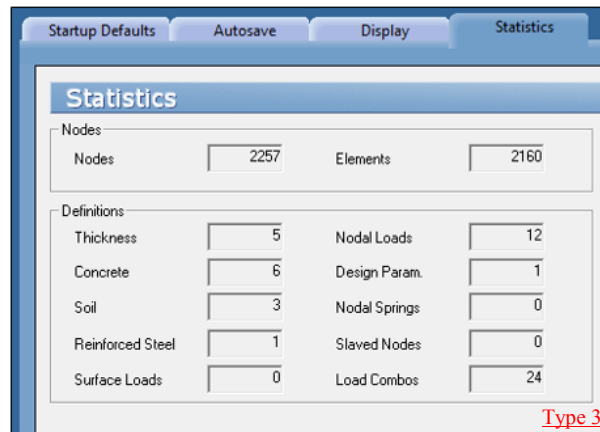
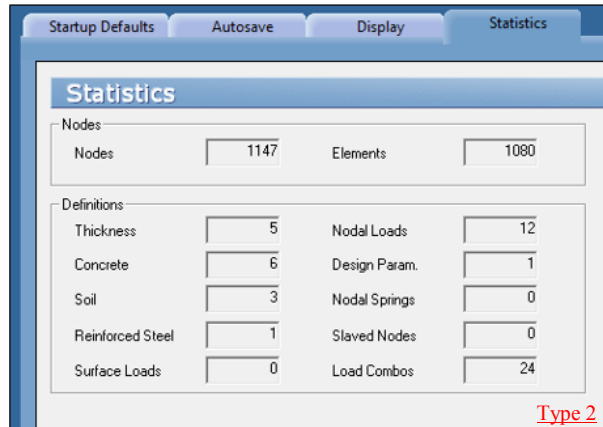
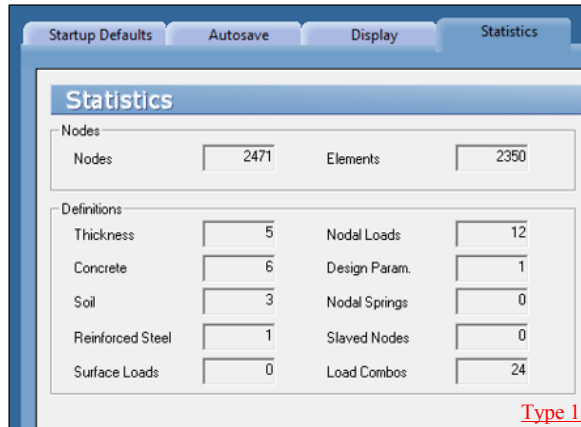


Figure 15 – Model Statistics

5. Column and Pile Design - spColumn

spMats provides the options to export columns and piles information from the foundation model to spColumn. Input (CTI) files are generated by spMats to include the section, materials, and the loads from the foundation model required by spColumn for strength design and investigation of piles and columns. Once the foundation model is completed and successfully executed, the following steps illustrate the design of a sample pile and column.

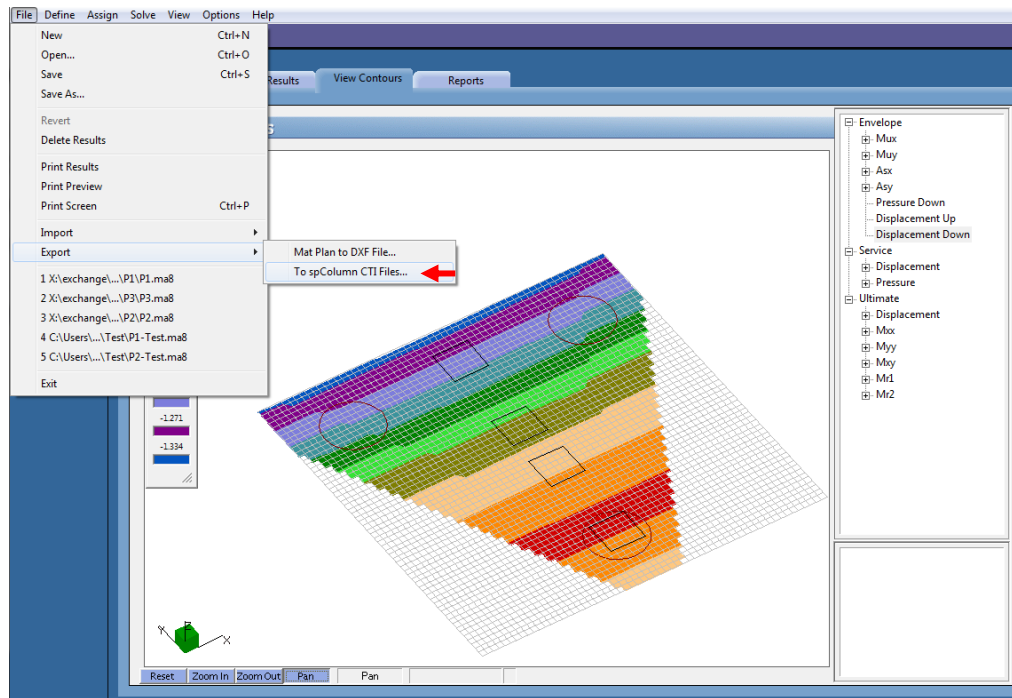


Figure 16 – Exporting CTI Files

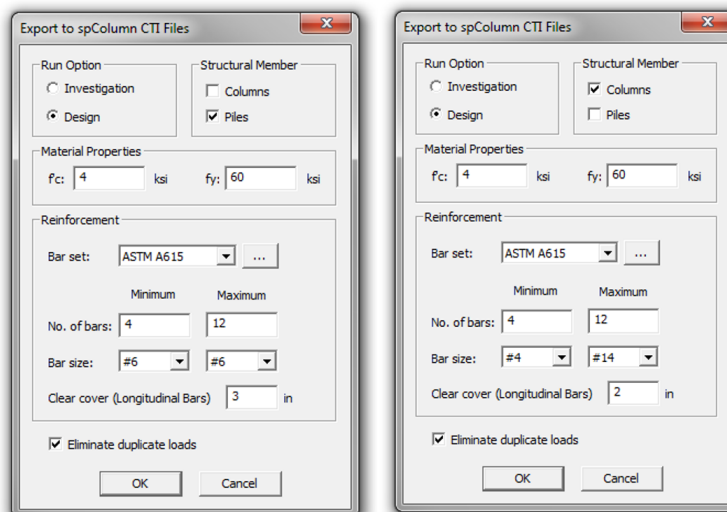


Figure 17 – Exporting CTI Files Dialog Box

After exporting **spColumn** input files, the pile and column design/investigation can proceed/modified to meet project specifications and criteria. In the following a sample pile and column design results are shown as an example.

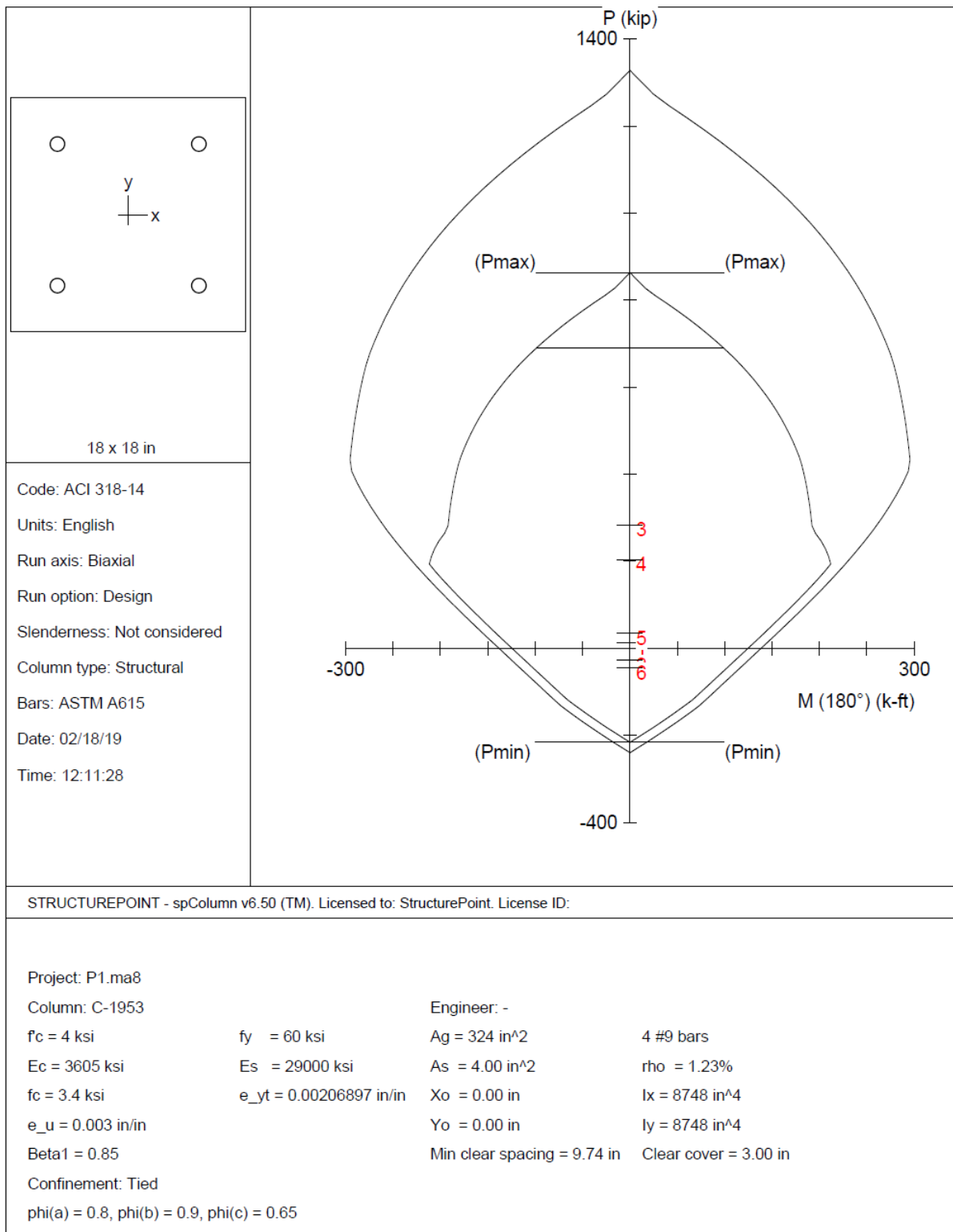


Figure 18 – Pier Interaction Diagram with Factored Load

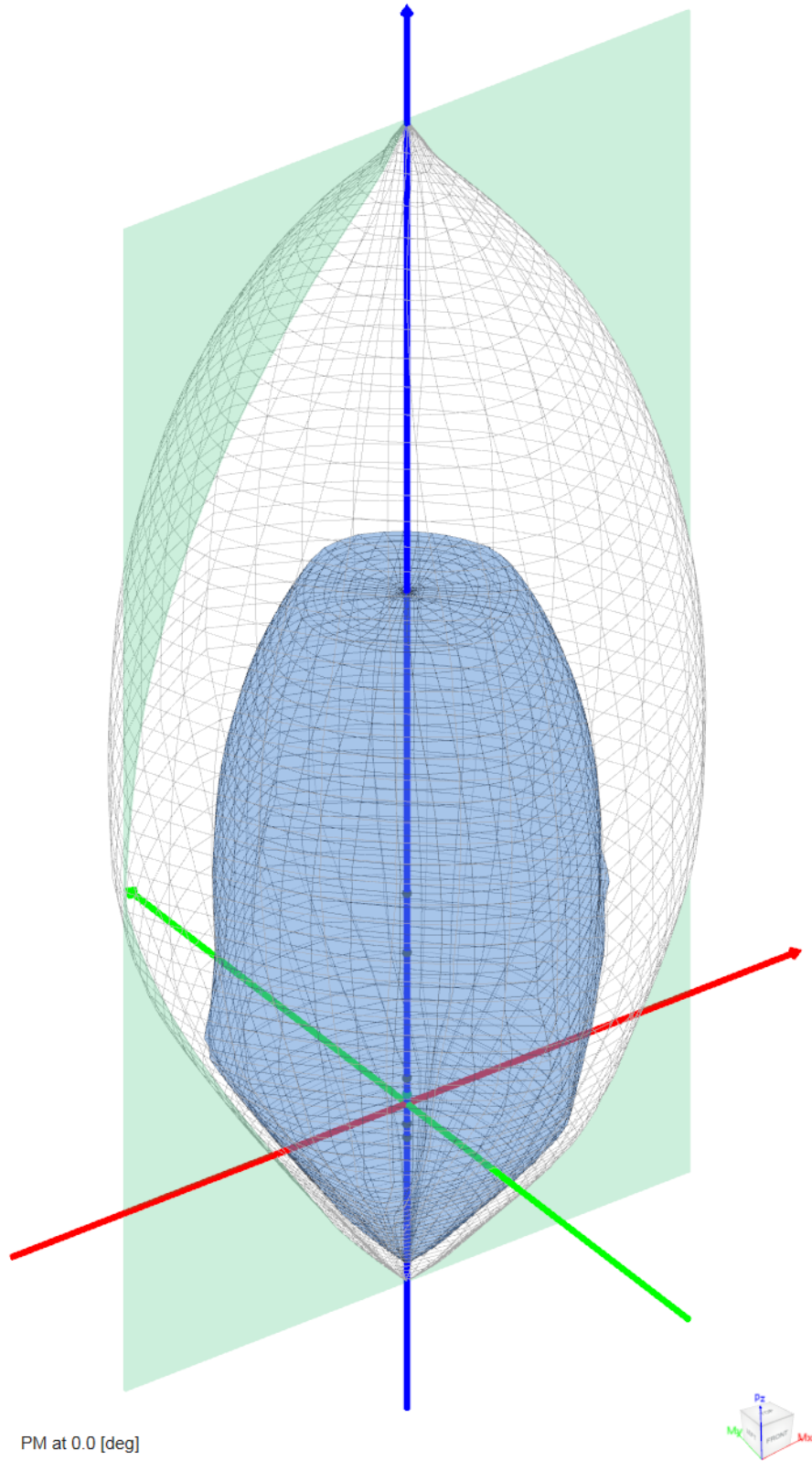


Figure 19 – Column 3D Failure Surfaces

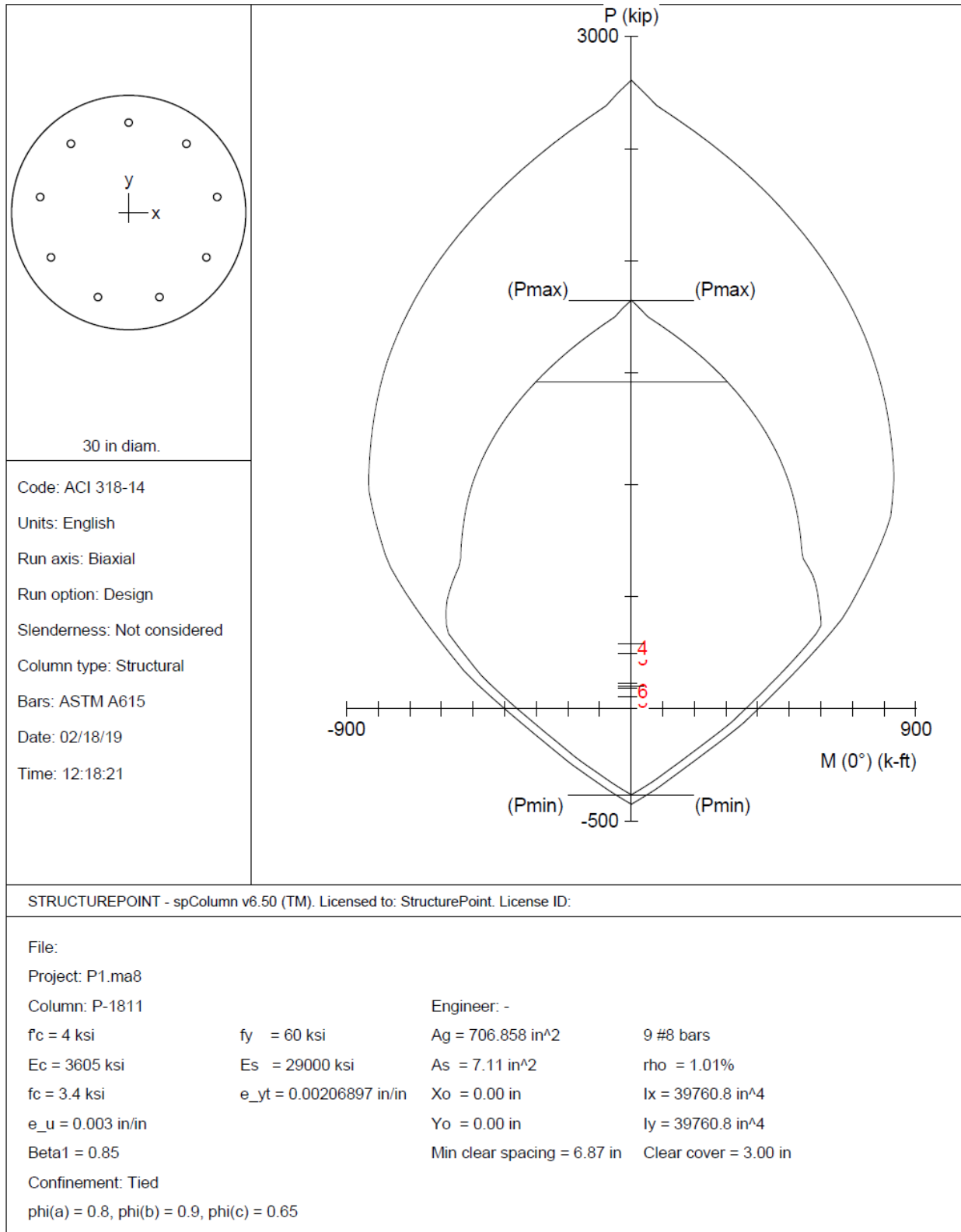


Figure 20 – Pile Interaction Diagram with Reaction Applied

6. 2D/3D Viewer

2D/3D Viewer is an advanced module of the [spColumn](#) program. It enables the user to view and analyze 2D interaction diagrams and contours along with 3D failure surfaces in a multi viewport environment.

2D/3D Viewer is accessed from within [spColumn](#). Once a successful run has been performed, you can open 2D/3D Viewer by selecting the **2D/3D Viewer** command from the **View** menu. Alternatively, 2D/3D Viewer can also be accessed by clicking the 2D/3D Viewer button in the program toolbar.

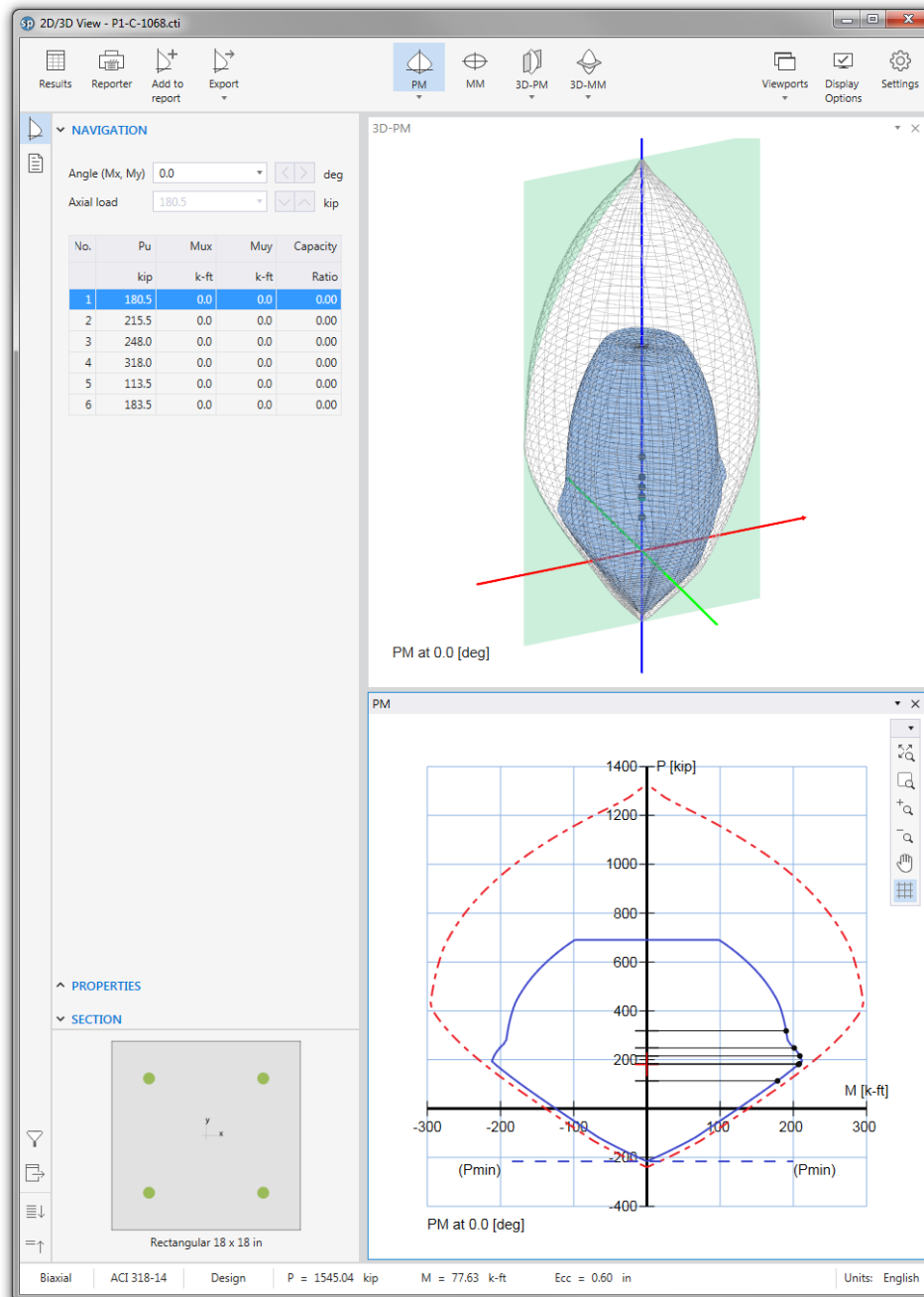


Figure 21 – 2D/3D View for Column

7. Tied vs. Spiral Confinement

The builder was provided two options for confinement to increase field and construction flexibility. The impact of spiral vs tied confinement is illustrated below. Note that pile diameter can be reduced below 30 inches and reactions reevaluated.

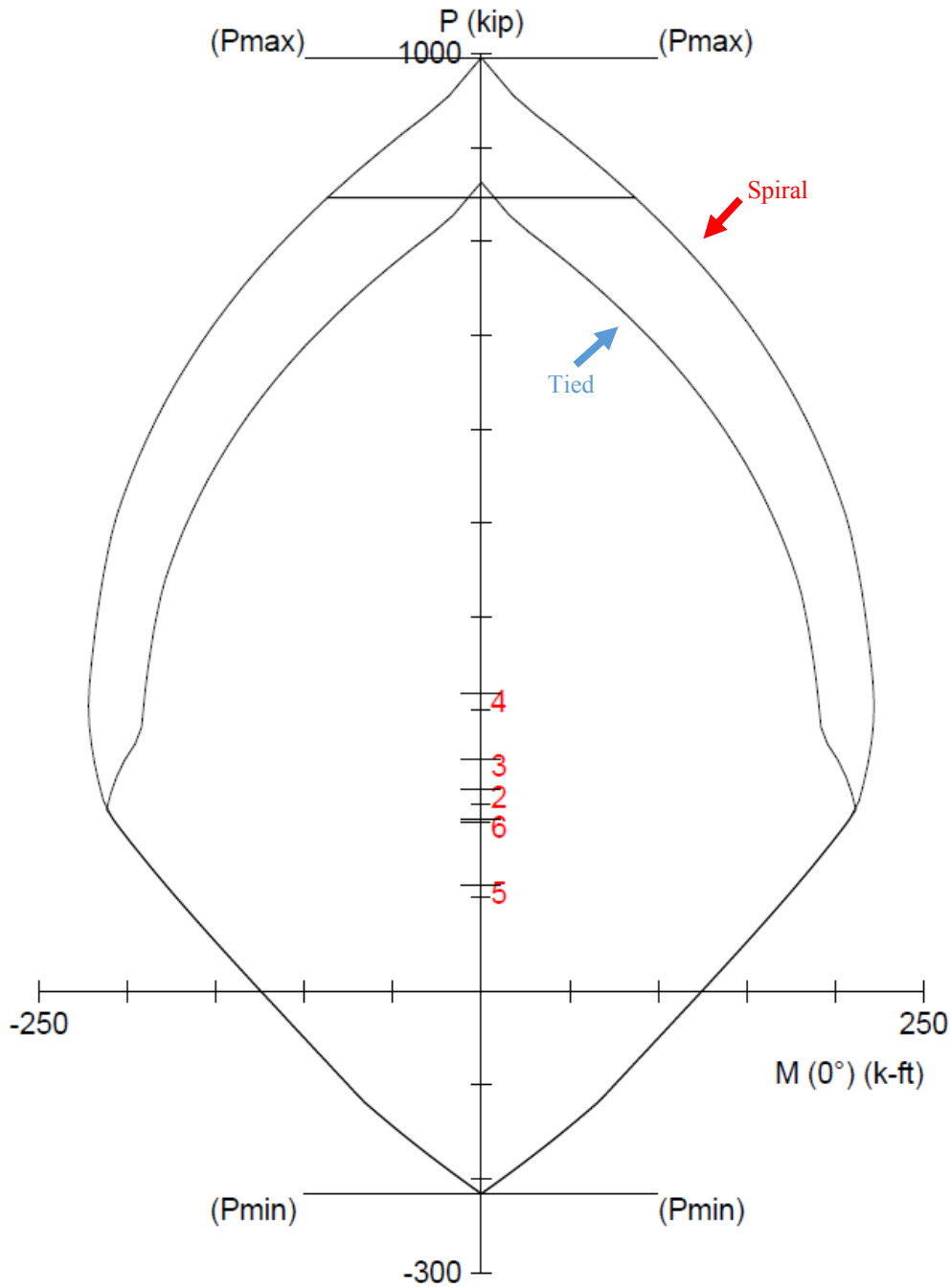


Figure 22 – Tied vs. Spiral Confinement